DESIGN LIVE LOADS			
FLOOR LIVE LOAD		<u>GENER/</u> 1.	AL THE REQUIREMENTS OF THESE GENERAL NOTES APPLY UNLESS OTHERWISE NOTED ON PLANS OR IN
OFFICES	**60 PSF		SPECIFICATIONS.
	100 PSF	2.	THE STRUCTURE HAS BEEN DESIGNED FOR A FUTURE SINGLE STORY VERTICAL EXPANSION. THE FUTURE STRUCTURE IS ASSUMED TO BE STEEL JOIST AND METAL DECKING AND THE SUBDOPTING COLU
	80 PSF		WHICH ARE TO BE EXTENDED. TO SUPPORT THE ROOF STRUCTURE ARE NOTED ON THE ROOF PLANS.
**(PLUS NON-REDUCED 20 PSF PARTITION LOAD ALLOWANCE)		3.	THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS IN THE FIELD PRIOR TO COMMENCING WORK. THE
(LIVE LOADS ARE REDUCED PER KBC SECTION 1607.9)			ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCIES WHICH MAY EXIST.
ROOF LIVE LOAD	20 PSF MIN	4.	ANY DISCREPANCIES BETWEEN STRUCTURAL AND ARCHITECTURAL DRAWINGS SHALL BE BROUGHT TO ATTENTION OF THE ARCHITECT AND STRUCTURAL ENGINEER
ROOF SNOW LOAD		5.	DO NOT SCALE DRAWINGS.
GROUND SNOW LOAD	Pg = 15 PSF	6.	THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION AND IS THEREFORE DEPEN
	Ce = 1.0		UPON DIAPHRAGM ACTION OF THE ROOF DECK AND FLOOR SLAB AND ATTACHMENT TO THE SHEAR WA
	$G_{1} = 1.0$		FOR STABILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES. THE CONTRACTOR SHALL FURN INSTALL ALL NECESSARY BRACING REQUIRED TO PROPERLY CONSTRUCT THE BUILDING UNTIL THESE
RAIN ON SNOW SURCHARGE	Pr = 5 PSF		ELEMENTS ARE COMPLETE AND CAPABLE OF PROVIDING THIS SUPPORT.
FLAT-ROOF SNOW LOAD* ((Pf = 0.7CeCtlsPg + Pr > (I Pg))	Pf = 18.0 PSF	7.	THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED ON
SLOPED-ROOF SNOW LOAD* (Ps = Cs Pf)	Ps = 18.0 PSF		STRUCTURAL FRAMING. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN CAPACITY OF THE FR
*(INCREASE FOR DRIFTING PER ASCE 7-05, SECTION 7.7)		8	SHOP DRAWINGS MUST BE CHECKED AND STAMPED BY THE CONTRACTOR PRIOR TO SUBMISSION
		9.	NON-STRUCTURAL ELEMENTS OF THE BUILDING (ARCHITECTURAL FINISHES, MASONRY VENEER AND
WIND LOAD (PER ASCE 7-05 CHAPTER 6)			ASSOCIATED TIES, INSULATION, SHEATHING, DUCTWORK, PIPING, ETC.) ARE TYPICALLY NOT SHOWN OF
BASIC WIND SPEED	V = 90 MPH		STRUCTURAL DRAWINGS. WHERE NON-STRUCTURAL ELEMENTS ARE SHOWN ON THE STRUCTURAL DRAWINGS THEY ARE SHOWN FOR RECEPTION AND DESIGN INTENT ONLY. NON STRUCTURAL ELEMENT
	CATEGORY IV		SHALL BE CONSTRUCTED AS SHOWN ON THE ARCHITECTURAL ELECTRICAL AND PLUMBING DRAWINGS
	EXPOSURE C	10.	ELEVATIONS SHOWN ON STRUCTURAL DRAWINGS ARE IDEALIZED ELEVATIONS BASED ON DECK THICKI
	IW = 1.15		AND SLOPES SHOWN ON DRAWINGS AND DO NOT ACCOUNT FOR BEAM JOIST CAMBER. IT IS THE
MAIN WIND FORCE RESISTING SYSTEM			RESPONSIBILITY OF THE CONTRACTORS TO COORDINATE ANY CAMBER OF THEIR WORK WITH OTHER T AND AD JUST ELEVATIONS AS NECESSARY TO ACCOUNT FOR DEAD LOAD DEELECTION AND THIS CAMBE
HORIZONTAL LOADS		11.	WALL OPENINGS AND TERMINATIONS SHOWN ON THE STRUCTURAL DRAWINGS ARE DIAGRAMMATIC ON
	. 17.8 PSF		WALL TERMINATIONS AND OPENING JAMBS, HEADS, AND SILLS SHALL BE CONSTRUCTED AS SHOWN OF
	. 11.8 PSF		ARCHITECTURAL DRAWINGS. WHERE VENEERS WRAP JAMBS, DETAIL AND FABRICATE LINTELS TO BEAF
	-5.6 PSF		ARCHITECTURAL DRAWINGS DO NOT INCLUDE DETAILS FOR ANY OF THESE CONDITIONS. CONSULT WIT
VERTICAL LOADS			ARCHITECT FOR DIRECTION.
END ZONE WINDWARD ROOF.	21.4 PSF		
	22.2 PSF	<u>FOUNDA</u>	ATION CONSTRUCTION
	14.9 F31	1	
COMPONENTS AND CLADDING WIND DESIGN PRESSURES PER ASCE 7-05, FIGURE 6-3		1.	4000 PSF. A GEOTECHNICAL REPORT WAS NOT AVAILABLE AT THE TIME THE CONSTRUCTION DOCUMEN
ZONE 1	-20.3 PSF		ISSUED FOR BIDS.
ZUNE 2	33.9 PSF	2.	ELEVATIONS GIVEN ARE TO THE TOP OF FOOTINGS AND GRADE BEAMS.
ZONE 3	±33.3 1 31 ±22.0 PSF	5.	APPRECIABLE SETTLEMENT.
ZONE 5	±27.1 PSF	4.	LOCATE EXISTING UNDERGROUND UTILITIES IN AREAS OF CONSTRUCTION. COORDINATED WITH UTILIT
		F	COMPANIES FOR ANY SHUT-OFF REQUIREMENTS OF STILL ACTIVE LINES.
EARTHQUAKE DESIGN DATA		Э.	ACCEPTABLE DEWATERING SYSTME SO THAT THE WATER LEVEL IS MAINTAINED CONTINUOUSLY A MINI
OCCUPANCY CATEGORY			0" BELOW THE EXCAVATION.
SITE CLASS	CLASS C	6.	BEFORE BACKFILL, ALL WALLS MUST BE ADEQUATELY BRACED. FOR BACKFILL REQUIREMENTS, SEE
MAPPED SHORT PERIOD RESPONSE ACCELERATION	Ss = 0.215	7	FOR PLACEMENT AND COMPACTION OF FILL LINDER SLAPS ON GRADE SEE SPECIFICATIONS. IF NOT O
	S1 = 0.091		NOTED, COMPACT ALL FILL TO 98% OF OPTIMUM LABORATORY DENSITY IN ACCORDANCE WITH ASTM D
DESIGN SHORT PERIOD SPECTRAL RESPONSE COEFFICIENT	SOS = 0.172 Sd1 = 0.103		STANDARD PROCTOR METHOD. PLACE FILL IN 6" TO 8" LAYERS AND COMPACT WITH VIBRATORY TAMPII
SEISMIC DESIGN CATEGORY	CATEGORY C	0	EQUIPMENT.
IMPORTANCE FACTOR	le = 1.5	δ.	DOWN SUBGRADE APPROPRIATELY FOR CONDUIT TO BE BELOW SLAB. COVER CONDUIT WITH FLOWAR
BASIC STRUCTURAL SYSTEM	BEARING WALL SYSTEM		(LEAN CONCRETE) TO BOTTOM OF SLAB ELEVATION.
SEISMIC RESISTING SYSTEM ORDINARY	REINFORCED CONCRETE		
RESPONSE MODIFICATION FACTOR	SHEAK WALLS $R = 4.0$		D PIER CONSTRUCTION
SEISMIC RESPONSE COEFFICIENT	Cs = 0.065	DINILLL	
METHOD OF ANALYSIS	ERAL FORCE PROCEDURE	1.	IF SUITABLE BEARING STRATA FOR SHALLOW FOOTINGS ARE NOT ENCOUNTERED, DRILLED PIERS MAY
SEISMIC BASE SHEAR(BASE BID)	. = 60.6 KIPS		INCLUDE UNIT COSTS AND PAYMENT WILL BE MADE BASED ON THE UNIT COSTS AND DEPTH OF PIER AN
			CLASSIFICATION OF EXCAVATION PROVIDED BY THE GEOTECHNICAL ENGINEER.
		2.	ALL DRILLED PIERS SHALL BEAR LEVEL WITH A MINIMUM EMBEDMENT OF 1'-0" INTO HARD LIMESTONE.
DESIGN STRESSES		3.	ALL DRILLED PIERS SHALL BE INSTALLED FROM THE LEVEL EXISTING AFTER GENERAL EXCAVATION HA
			DRILLED PIERS SHALL BE SEALED OFF TO PREVENT ANY INFLOW OF WATER SHALL BE SEALED OFF TO PREVENT ANY INFLOW OF WATER SHALL BE SEALED OFF TO PREVENT ANY INFLOW OF WATER SHALL BE SEALED OFF TO PREVENT ANY INFLOW OF WATER SHALL BE SEALED OFF TO PREVENT ANY INFLOW OF WATER SHALL BE SEALED OFF TO PREVENT ANY INFLOW OF WATER SHALL BE SEALED OFF TO PREVENT ANY INFLOW OF WATER SHALL BE SEALED OFF TO PREVENT ANY INFLOW OF WATER SHALL BE SEALED OFF TO PREVENT ANY INFLOW OF WATER SHALL BE SEALED OFF TO PREVENT ANY INFLOW OF WATER SHALL BE SAND. OR SHALL BE SEALED OFF TO PREVENT ANY INFLOW OF WATER SHALL BE SAND. OR SHALL BE SEALED OFF TO PREVENT ANY INFLOW OF WATER SHALL BE SAND. OR SHALL BE SAND.
FOOTINGS AND DRILLED PIERS	f'c = 3.000 PSI		DURING INSTALLATION. SUBMIT PROPOSED DRILLED PIER INSTALLATION METHOD TO CONSTRUCTION
INTERIOR SLABS ON GRADE, GRADE BEAMS AND ICF WALLS	f'c = 4,000 PSI		MANAGER AND ENGINEER FOR REVIEW PRIOR TO START OF ANY DRILLED PIER WORK.
CONCRETE SUBJECT TO DE-ICING SALTS	f'c = 4,500 PSI	4.	ALL DRILLED PIERS SHALL BE INSPECTED DURING DRILLING AND PLACING OF CONCRETE. A COMPLETE
REINFORCING BARS (ASTM A615 GRADE 60)	fy = 60,000 PSI		OF EACH DRILLED FIER INSTALLED SHALL BE MADE. SEE SPECIFICATIONS FOR REQUIREMENTS OF INS AND REPORT.
WELDED WIRE FABRIC (ASTM A185)	fy = 65,000 PSI	5.	ALL DRILLED PIERS SHALL BE FILLED WITH 4000 PSI CONCRETE (28 DAY STRENGTH). NO CONCRETE SH
WIDE FLANGE AND TEE SHAPES DESIGNATED AS WAND WT (ASTM A992)	fy = 50,000 PSI		PLACED UNTIL THE DRILLED PIER HAS BEEN INSPECTED AND APPROVED. AN ACCURATE VOLUME CHEC
WIDE FLANGE AND THE SHAPES DESIGNATED AS M, S, HP, MT AND ST (ASTM 36)	Ty = 36,000 PSI		CONCRETE PLACED IN EACH DRILLED PIER SHALL BE MADE BY THE CONTRACTOR AS A CHECK TO VERI
HULLUW STRUCTURAL SECTIONS - RECTAINGULAR STEEL TUBES(ASTM A500 GRADE 50) CHANNELS, ANGLES, PLATES AND BARS (ASTM A36)	iy = 42,000 PSI fv = 36,000 PSI		CLEANOUT OF THE EXCAVATION.
ASSUMED SOIL BEARING PRESSURE FOR FOUNDATIONS (BEDROCK).	4.000 PSF	6.	CENTERLINES OF ALL DRILLED PIER SHAFTS SHALL BE CHECKED FOR CONFORMANCE TO SPECIFICATION
	.,		TO PLACING CONCRETE. ANY DEVIATION FROM THE ACCEPTABLE INSTALLATION TOLERANCES SHALL
			REPORTED TO THE ENGINEER AND NO CONCRETE SHALL BE PLACED UNTIL AUTHORIZED BY THE ENGINARY CORRECTIVE MEASURES REQUIRED RECAUSE OF MISALIGNMENT REVOND THE PERMISSIBLE TO F
			THE RESPONSIBILITY OF THE DRILLED PIER CONTRACTOR.

GENERAL NOTES

CONCRETE CONSTRUCTION

- 1. ALL CONCRETE CONSTRUCTION TO BE IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE ACI 301-05, ACI 318-05 AND ACI DETAILING MANUAL. FURNISH BAR SUPPORTS WHERE NECESSARY DURING CONSTRUCTION.
- ALL REINFORCING BARS SHALL BE SECURELY TIED PRIOR TO PLACING CONCRETE. PROVIDE PIPE SLEEVES AND INSERTS IN CONCRETE WORK WHERE REQUIRED. SEE ARCHITECTURAL AND
- MECHANICAL DRAWINGS. CONSTRUCTION JOINTS SHALL BE POSITIONED SO AS NOT TO CHANGE THE STRUCTURAL DESIGN
- REQUIREMENTS. LOCATION OF ALL CONSTRUCTION JOINTS SHALL BE APPROVED BY THE ENGINEER. WELDING OF REINFORCING BARS (INCLUDING TACK WELDING) IS NOT PERMITTED.
- PROVIDE HORIZONTAL KEYWAYS IN CONSTRUCTION JOINTS IN GRADE BEAMS. SEE SCHEDULE FOR REQUIREMENTS. ALL EXPOSED CORNERS OF CONCRETE SHALL BE CHAMFERED 45 DEGREES. MINIMUM CHAMFER TO BE 1/2".
- CURVE THE LEADING EDGE OF STAIR TREADS TO 1/2" RADIUS. REINFORCING FOR SLABS ON GROUND (IN FLAT SHEETS) SHALL BE IN THE MIDDLE OF THE SLAB EXCEPT AS OTHERWISE NOTED AND SHALL BE POSITIVELY SUPPORTED AND MAINTAINED IN THIS POSITION DURING
- PLACEMENT OF CONCRETE. 10. BEND ALL HORIZONTAL WALL 1'-0" AROUND CORNERS OR PROVIDE CORNER BARS WITH
- 2'-0" LAP. 11. PROVIDE FOUNDATION DOWELS FOR ALL WALLS, AND PIERS, SAME SIZE AND SPACING AS
- VERTICAL STEEL. 12. SPLICES:

A. LAP ALL COMPRESSION SPLICES 30 BAR DIAMETERS OF THE LARGER BAR. B. LAP ALL TENSION SPLICES IN ACCORDANCE WITH THE FOLLOWING TABLE. MODIFY LENGTHS AS NOTED:

BAR SIZE	CONCRETE	COMPRESSIVE	STRENGTH	1. INCREASE SPLICE LENGTH BY THE FOLLOWING:		
	3,000 PSI	4,000 PSI	5,000 PSI	2. NOTE: INCREASED LENGTHS ARE ACCUN	IULATIVE	
#3	21"	19"	17"	1. HORIZONTAL TOP BARS WITH GREATER	ATER	
#4	29"	25"	22"	2. BAR SPACING LESS THAN 2 BAR	+30%	
#5	36"	31"	28"	DIAMETERS	+50%	
#6	43"	37"	33"			
#7	62"	54"	48"			
#8	71"	62"	55"			

13. CONCRETE PROTECTION FOR REINFORCEMENT: COVER A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH . B. CONCRETE EXPOSED TO EARTH OR WEATHER NO. 6 THROUGH NO. 18 BARS ... NO. 5 BAR, W31 OR D31 WIRE AND SMALLER . 1 1/2" C. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND SLABS AND WALLS NO. 11 BAR AND SMALLER . 3/4

PRECAST CONCRETE HOLLOW CORE SLABS

- 1. PROVIDE HIGH-DENSITY PLASTIC BEARING PADS UNDER ALL PRECAST BEARING ON CONCRETE. 2. PRECAST MANUFACTURER SHALL DESIGN AND FURNISH ALL HEADERS REQUIRED AT OPENINGS THROUGH
- SI ABS 3. HOLES SHALL NOT BE FIELD CUT THROUGH PRECAST CORES WITHOUT APPROVAL OF THE ENGINEER. 4. REFER TO STRUCTURAL LEGEND FOR DESIGN LOADS.

RO<u>OF, FLOOR, OR WALL OPENINGS</u>

- 1. THE CONTRACTOR SHALL VERIFY AND COORDINATE THE NUMBER, SIZE, AND LOCATION OF ALL SLEEVES
- AND OPENINGS REQUIRED FOR MECHANICAL OR ELECTRICAL ITEMS. 2. SLEEVES AND OPENINGS SHALL BE LOCATED IN A MANNER THAT WILL MAINTAIN THE STRUCTURAL
- INTEGRITY OF THE ROOF, FLOOR, OR WALL SYSTEM. 3. NO STRUCTURAL ELEMENTS ARE TO BE CUT UNLESS SPECIFICALLY APPROVED BY THE ENGINEER.

- UCTURE HAS BEEN DESIGNED FOR A FUTURE SINGLE STORY VERTICAL EXPANSION. THE FUTURE RUCTURE IS ASSUMED TO BE STEEL JOIST AND METAL DECKING AND THE SUPPORTING COLUMNS RE TO BE EXTENDED, TO SUPPORT THE ROOF STRUCTURE ARE NOTED ON THE ROOF PLANS. TRACTOR SHALL VERIFY ALL DIMENSIONS IN THE FIELD PRIOR TO COMMENCING WORK. THE R SHALL BE NOTIFIED OF ANY DISCREPANCIES WHICH MAY EXIST. CREPANCIES BETWEEN STRUCTURAL AND ARCHITECTURAL DRAWINGS SHALL BE BROUGHT TO THE ON OF THE ARCHITECT AND STRUCTURAL ENGINEER. UCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION AND IS THEREFORE DEPENDENT APHRAGM ACTION OF THE ROOF DECK AND FLOOR SLAB AND ATTACHMENT TO THE SHEAR WALLS
- BILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES. THE CONTRACTOR SHALL FURNISH AND ALL NECESSARY BRACING REQUIRED TO PROPERLY CONSTRUCT THE BUILDING UNTIL THESE IS ARE COMPLETE AND CAPABLE OF PROVIDING THIS SUPPORT. TRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED ON THE JRAL FRAMING. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN CAPACITY OF THE FRAMING IME THE LOADS ARE IMPOSED.
- AWINGS MUST BE CHECKED AND STAMPED BY THE CONTRACTOR PRIOR TO SUBMISSION. UCTURAL ELEMENTS OF THE BUILDING (ARCHITECTURAL FINISHES, MASONRY VENEER AND TED TIES, INSULATION, SHEATHING, DUCTWORK, PIPING, ETC.) ARE TYPICALLY NOT SHOWN ON THE JRAL DRAWINGS. WHERE NON-STRUCTURAL ELEMENTS ARE SHOWN ON THE STRUCTURAL GS, THEY ARE SHOWN FOR REFERENCE AND DESIGN INTENT ONLY. NON-STRUCTURAL ELEMENTS E CONSTRUCTED AS SHOWN ON THE ARCHITECTURAL, ELECTRICAL AND PLUMBING DRAWINGS. DNS SHOWN ON STRUCTURAL DRAWINGS ARE IDEALIZED ELEVATIONS BASED ON DECK THICKNESSES PES SHOWN ON DRAWINGS AND DO NOT ACCOUNT FOR BEAM JOIST CAMBER. IT IS THE SIBILITY OF THE CONTRACTORS TO COORDINATE ANY CAMBER OF THEIR WORK WITH OTHER TRADES UST ELEVATIONS AS NECESSARY TO ACCOUNT FOR DEAD LOAD DEFLECTION AND THIS CAMBER. ENINGS AND TERMINATIONS SHOWN ON THE STRUCTURAL DRAWINGS ARE DIAGRAMMATIC ONLY.
- RMINATIONS AND OPENING JAMBS, HEADS, AND SILLS SHALL BE CONSTRUCTED AS SHOWN ON THE CTURAL DRAWINGS. WHERE VENEERS WRAP JAMBS, DETAIL AND FABRICATE LINTELS TO BEAR ON RUCTURE. DO NOT BEAR LINTELS OR BEAMS ON VENEERS (BRICKS, SIDING, ETC.). IF THE CTURAL DRAWINGS DO NOT INCLUDE DETAILS FOR ANY OF THESE CONDITIONS, CONSULT WITH
- FIONS FOR THIS PROJECT ARE DESIGNED WITH AN ASSUMED ALLOWABLE ROCK BEARING PRESSURE OF . A GEOTECHNICAL REPORT WAS NOT AVAILABLE AT THE TIME THE CONSTRUCTION DOCUMENTS WERE DNS GIVEN ARE TO THE TOP OF FOOTINGS AND GRADE BEAMS.
- TINGS MUST BE SUPPORTED ON SOUND BEDROCK CAPABLE OF SUPPORTING DESIGN LOADS WITHOUT EXISTING UNDERGROUND UTILITIES IN AREAS OF CONSTRUCTION. COORDINATED WITH UTILITY IES FOR ANY SHUT-OFF REQUIREMENTS OF STILL ACTIVE LINES. CAVATIONS APPROACH THE GROUND WATER LEVEL, THE WATER LEVEL SHALL BE LOWERED BY AN BLE DEWATERING SYSTME SO THAT THE WATER LEVEL IS MAINTAINED CONTINUOUSLY A MINIMUM OF 2'-BACKFILL, ALL WALLS MUST BE ADEQUATELY BRACED. FOR BACKFILL REQUIREMENTS, SEE ATIONS AND/OR GEOTECHNICAL ENGINEER'S REPORT.
- CEMENT AND COMPACTION OF FILL UNDER SLABS ON GRADE, SEE SPECIFICATIONS. IF NOT OTHERWISE COMPACT ALL FILL TO 98% OF OPTIMUM LABORATORY DENSITY IN ACCORDANCE WITH ASTM D698 RD PROCTOR METHOD. PLACE FILL IN 6" TO 8" LAYERS AND COMPACT WITH VIBRATORY TAMPING LECTRICAL CONDUIT CONGREATES BELOW ELECTRICAL ROOMS AND PANELS, CONTRACTOR SHALL HOLD
- JBGRADE APPROPRIATELY FOR CONDUIT TO BE BELOW SLAB. COVER CONDUIT WITH FLOWABLE FILL NCRETE) TO BOTTOM OF SLAB ELEVATION.

BLE BEARING STRATA FOR SHALLOW FOOTINGS ARE NOT ENCOUNTERED, DRILLED PIERS MAY BE ED. THE REQUIRED DEPTHS SHALL BE DETERMINED BY THE GEOTECHNICAL ENGINEER. BIDS SHALL UNIT COSTS AND PAYMENT WILL BE MADE BASED ON THE UNIT COSTS AND DEPTH OF PIER AND CATION OF EXCAVATION PROVIDED BY THE GEOTECHNICAL ENGINEER. LED PIERS SHALL BEAR LEVEL WITH A MINIMUM EMBEDMENT OF 1'-0" INTO HARD LIMESTONE. LED PIERS SHALL BE INSTALLED FROM THE LEVEL EXISTING AFTER GENERAL EXCAVATION HAS SED TO EXTENT REQUIRED. SEE SPECIFICATIONS FOR TOLERANCES IN DRILLED PIER INSTALLATION. PIERS SHALL BE SEALED OFF TO PREVENT ANY INFLOW OF WATER. SILT. SAND. OR SIMILAR MATERIALS NSTALLATION. SUBMIT PROPOSED DRILLED PIER INSTALLATION METHOD TO CONSTRUCTION R AND ENGINEER FOR REVIEW PRIOR TO START OF ANY DRILLED PIER WORK. LED PIERS SHALL BE INSPECTED DURING DRILLING AND PLACING OF CONCRETE. A COMPLETE REPORT DRILLED PIER INSTALLED SHALL BE MADE. SEE SPECIFICATIONS FOR REQUIREMENTS OF INSPECTION LED PIERS SHALL BE FILLED WITH 4000 PSI CONCRETE (28 DAY STRENGTH), NO CONCRETE SHALL BE UNTIL THE DRILLED PIER HAS BEEN INSPECTED AND APPROVED. AN ACCURATE VOLUME CHECK OF TE PLACED IN EACH DRILLED PIER SHALL BE MADE BY THE CONTRACTOR AS A CHECK TO VERIFY THE MENSIONS OF THE DRILLED PIER. CONCRETE SHALL BE PLACED THE SAME DAY AS THE FINAL UT OF THE EXCAVATION. LINES OF ALL DRILLED PIER SHAFTS SHALL BE CHECKED FOR CONFORMANCE TO SPECIFICATION PRIOR

NG CONCRETE. ANY DEVIATION FROM THE ACCEPTABLE INSTALLATION TOLERANCES SHALL BE ED TO THE ENGINEER AND NO CONCRETE SHALL BE PLACED UNTIL AUTHORIZED BY THE ENGINEER. RECTIVE MEASURES REQUIRED BECAUSE OF MISALIGNMENT BEYOND THE PERMISSIBLE TOLERANCE IS PONSIBILITY OF THE DRILLED PIER CONTRACTOR. 7. THE LOCATION OF EXISTING SEWER AND UNDERGROUND UTILITIES SHALL BE VERIFIED PRIOR TO THE DRILLING OF DRILLED PIERS. IF NEW DRILLED PIERS WILL ENCROACH UPON THOSE EXISTING STRUCTURES. THE CONTRACTOR SHALL CONTACT THE ENGINEER FOR DIRECTION PRIOR TO PLACING DRILLED PIERS.

SHAFTS SHALL BE CASED AND DEWATERED PRIOR TO INSPECTION AND CONCRETE PLACEMENT.

STEEL CONSTRUCTION

- 1. STEEL DETAILING, FABRICATION, AND ERECTION SHALL CONFORM TO THE AISC SPECIFICATIONS AND CODE OF STANDARD PRACTICE, AND THE AWS STRUCTURAL WELDING CODE.
- 2. CONNECTIONS WELDED OR HIGH STRENGTH BOLTED: A. A325-N WITH HARDENED WASHERS - USE FOR ALL CONNECTIONS B. ALL BOLTS SHALL BE TIGHTENED TO FULL PRETENSIONING LOAD.
- C. UNLESS SPECIFICALLY NOTED ON THE DRAWINGS OR WITHOUT WRITTEN PERMISSION FROM THE ENGINEER, ALL BOLTS FOR THE PROJECT SHALL BE OF ONE ASTM TYPE AND ONE DIAMETER.
- D. USE STANDARD HOLES WITH THE FOLLOWING EXCEPTIONS: SHORT SLOTTED HOLES ARE PERMITTED FOR SHEAR LOADING PERPENDICULAR TO THE SLOT.
- E. HARDENED WASHERS SHALL BE USED OVER ALL OVERSIZED OR SHORT-SLOTTED HOLES IN AN OUTER PLY. WHERE LONG-SLOTTED HOLES ARE USED IN AN OUTER PLY, 5/16" THICK A36 PLATE WASHERS OR CONTINUOUS BAR WITH STANDARD HOLES SHALL BE PROVIDED.
- F. WHERE REACTION IS NOTED, DEVELOP SAME. WHERE NOT NOTED, CONNECTIONS SHALL DEVELOP ONE-HALF OF THE TOTAL UNIFORM LOAD CAPACITY OF THE BEAM.
- G. PREAPPROVED CONNECTION DETAILS ARE PROVIDED ON DRAWING S-4.1 H. SINGLE PLATE SHEAR CONNECTIONS ARE NOT PERMITTED WHERE THE REACTION EXCEEDS 50 KIPS, AT FIELD APPLIED CONNECTIONS, OR CONNECTIONS TO COLUMNS (OTHER THAN AT SKEWED CONNECTIONS, MOMENT CONNECTIONS, PIPE COLUMNS, TUBE COLUMNS WITH FACE DIMENSION 4" OR LESS, OR CONNECTIONS WITH REACTIONS LESS THAN 15 KIPS). J. THROUGH PLATE CONNECTIONS AT TUBE COLUMNS ARE NOT PERMITTED, UNLESS NOTED OTHERWISE.
- SHEAR CONNECTIONS TO TUBE COLUMNS SHALL BE WT OR DOUBLE ANGLE KNIFE CONNECTIONS EXCEPT AS NOTED ABOVE.
- K. SHEAR CONNECTIONS TO VERTICAL EMBED PLATES IN CONCRETE WALLS SHALL BE DOUBLE ANGLE TYPE. 3. WELDING ELECTRODES SHALL BE E70XX EXCEPT WHERE OTHER ELECTRODES ARE REQUIRED FOR COMPATIBILITY WITH MATERIAL BEING WELDED.
- 4. SHOP DRAWINGS ARE REQUIRED AND SHALL NOTE TYPE OF ELECTRODES, SIZE OF ALL WELDS, AND TYPE AND SIZE OF ALL BOLTS.
- 5. SEE SPECIFICATIONS FOR ALL PAINTING REQUIREMENTS. 6. ALL SHOP AND FIELD WELDING SHALL BE DONE BY A CERTIFIED WELDER.

SECTIONS AND THROUGH THE SHEAR CENTER OF CHANNELS.

7. PROVIDE MINIMUM OF (4) 3/4" DIAMETER x 1'-0" EMBED ANCHOR BOLTS AND 1 1/2" GROUT UNDER ALL COLUMN BASE PLATES. 8. MISCELLANEOUS HANGING LOADS SUCH AS STAIR STRINGERS, PIPES, MECHANICAL UNITS, ETC., SUPPORTED BY STEEL MEMBERS SHALL HAVE THESE LOADS APPLIED IN SUCH A MANNER THAT NO TORSIONAL FORCES ARE INDUCED IN THESE MEMBERS, I.E., LOADS SHALL PASS THROUGH THE CENTERLINE OF WIDE FLANGE

SPECIAL INSPECTION

- 1. SPECIAL INSPECTIONS AS DEFINED IN SECTION 1704.0 OF THE KENTUCKY BUILDING CODE ARE REQUIRED. 2. SPECIAL INSPECTIONS SHALL BE PERFORMED BY A QUALIFIED TESTING AGENCY APPROVED BY THE ARCHITECT
- AND THE STRUCTURAL ENGINEER AND PAID FOR BY THE OWNER. 3. THE INSPECTOR SHALL OBSERVE WORK FOR CONFORMANCE WITH THE APPROVED STRUCTURAL DRAWINGS
- AND SPECIFICATIONS AND PREPARE INSPECTION REPORTS STATING HIS/HER OBSERVATIONS. COPIES OF THE INSPECTION REPORTS SHALL BE SUBMITTED TO THE CONTRACTOR, THE ARCHITECT AND THE STRUCTURAL
- ENGINEER. 4. ALL DISCREPANCIES BETWEEN THE CONSTRUCTION DOCUMENTS AND THE WORK BEING PERFORMED SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THE DISCREPANCIES ARE
- NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF ARCHITECT AND THE STRUCTURAL ENGINEER PRIOR TO THE COMPLETION OF THAT PHASE OF THE WORK.
- 5. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL REPORT OF INSPECTIONS DOCUMENTING COMPLETION OF ALL REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS. 6. SPECIAL INSPECTIONS ARE REQUIRED FOR THE FOLLOWING WORK:

<u>INSPECTION OF FABRICATORS</u> PERFORM SPECIAL INSPECTIONS PER SECTION 1704.2 OF THE KENTUCKY BUILDING CODE.
STEEL CONSTRUCTION PERFORM SPECIAL INSPECTIONS PER SECTION 1704.3 OF THE KENTUCKY BUILDING CODE.
<u>CONCRETE CONSTRUCTION</u> PERFORM SPECIAL INSPECTIONS PER SECTION 1704.4 OF THE KENTUCKY BUILDING CODE.
SOILS CONSTRUCTION PERFORM SPECIAL INSPECTIONS PER SECTION 1704.7 OF THE KENTUCKY BUILDING CODE.

DRILLED PIER CONSTRUCTION PERFORM SPECIAL INSPECTIONS PER SECTION 1704.8 OF THE KENTUCKY BUILDING CODE.

STRUCTURAL ABBREVIATIONS

APA	AMERICAN PLYWOOD ASSOCIATION	I.C.F.	INSUL
ARCH	ARCHITECTURAL	LLH	LONG
BOT	BOTTOM	LLV	LONG
CLR	CLEAR	LFH	LONG
CANT	CANTILEVER BEAM	LFV	LONG
C.M.U.	CONCRETE MASONRY UNIT	LVL	LAMIN
CONT	CONTINUOUS	MAX	MAXIN
D	DEEP	MIN	MINIM
DET	DETAIL	N.T.S.	NOT T
DWGS	DRAWINGS	0.C.	ON CE
E.F.	EACH FACE	OPH	OPPO
ELEV	ELEVATION	P.A.F.	POWE
E.W.	EACH WAY	PL	PLATE
E.O.S.	EDGE OF SLAB	R	RADIL
EXP	EXPANSION	SIM	SIMIL
F.F.E.	FINISHED FLOOR ELEVATION	S.O.G.	SLAB
F.R.C.	FIBER REINFORCED CONCRETE	TYP	TYPIC
GALV	GALVANIZED	VERT	VERTI
GA	GAUGE	W	WIDE
HORIZ	HORIZONTAL	W.W.F.	WELD
HSS	HOLLOW STRUCTURAL SECTION		

MATERIAL LEGEND



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