GENERAL NOTES

MASONRY WALL CONSTRUCTION (CONTINUED)

	<u>GN STRESSES</u> CRETE (STRENC	GTH DESIGN) M	IINIMUM COMP	RESSIVE STRE	NGTH IN 28 DAYS f'c	= 4,000 PSI	
REIN WELD	FORCING BARS DED WIRE FABR	(ASTM A615 G IC (ASTM A185)	RADE 60))		fy	= 60,000 PSI = 65,000 PSI	
WIDE CHAN	EFLANGE AND T NNELS, ANGLES	EE SHAPES DE 5, PLATES, AND	ESIGNATED AS BARS (ASTM A	W AND WT (AS (36)	STM A992)	= 50,000 PSI = 36,000 PSI	
COMF STEE	POSITE STEEL I L FORM DECK (DECK (ASTM A6 ASTM A653)	653)		fy	= 40,000 PSI = 60,000 PSI	
HOLL	OW STRUCTUR	AL SECTIONS - RY ASSEMBLY (- RECTANGULA	AR STEEL TUBE STRENGTH	S (ASTM 500 GRADE B) fy	= 46,000 PSI = 1,500 PSI	
	CONCRETE TYPE S MO	E MASONRY UN RTAR STRENG	IIT STRENGTH		f'm f'm	= 1,900 PSI = 1,800 PSI	
	MASONRY	GROUT (ASTM	C476) MINIMUN	M COMPRESSI\	/E STRENGTH IN 28 DAYS f'g	= 2,000 PSI	
<u>GENE</u>	ERAL						
1.]		ENTS OF THES	E GENERAL NO	OTES APPLY U	NLESS OTHERWISE NOTED ON		
2.]		OR SHALL VER	RIFY ALL DIMEN	SIONS IN THE	FIELD PRIOR TO COMMENCING		
3. / F	ANY DISCREPAI	NCIES BETWEE	IN STRUCTURA	AL AND ARCHIT	ECTURAL DRAWINGS SHALL BE		
4. [DO NOT SCALE	DRAWINGS.					
U. (CONTRACTOR S	SHALL FURNISH		ALL NECESSA	RY BRACING / SHORING	2	
6 5	COMPLETE. SUBMITTALS MI	JST BE CHECK				,	
S	SUBMISSION.						
CONC	CRETE CONSTR	<u>UCTION</u>					
1.	ALL CONCRETI	E CONSTRUCTI CONCRETE ACI	ION TO BE IN A 301-05, ACI 31	CCORDANCE V 8-05 AND ACI D	VITH THE BUILDING CODE REQU ETAILING MANUAL, EXCEPT TH	IREMENTS FOR AT CONSTRUCT	ION
2.	AND REMOVAL	. OF FORMS AN SUPPORTS WH	ID RESHORING IERE NECESSA	SHALL BE INS	PECTED BY THE CONTRACTOR	S ENGINEER.	
3.	PROVIDE PLAS	STIC, PLASTIC-C /IEW IN COMPL	COATED (NOT F ETED STRUCT	PLASTIC-TIPPE URE.	D) OR STAINLESS STEEL CHAIR	S IN ALL CONCR	RETE
4.	PROVIDE PIPE MECHANICAL I	SLEEVES AND DRAWINGS.	INSERTS IN CO				٧D
5.	CONSTRUCTIO REQUIREMENT	N JOINTS SHAI	LL BE POSITION	NED SO AS NO OFS SHALL HA	T TO CHANGE THE STRUCTURA	L DESIGN THAT NO MORE	
	I HAN 10,000 SO	U.FI. IS IN ANY	SINGLE POUR SHALL NOT EX	CEED 2. LOCA	INUM LENGTH OF POUR IS 100'- TION OF ALL CONSTRUCTION J	U". RATIO OF DINTS SHALL BE	
6.	APPROVED BY WELDING OF R		K. BARS (INCLUDIN		ING) IS NOT PERMITTED.		
1.	AND WALL FOO	DTINGS; MINIMU	ATS IN CONST JM 1 1/2" DEPT	H WITH HEIGH	TEQUAL TO ONE-THIRD OF MEN	LO, IBER DEPTH,	
8.		CORNERS OF (CONCRETE SH		ERED 45 DEGREES. MINIMUM C	HAMFER TO BE	1/2'
9.		FOR SLABS ON	N GROUND (IN I SHALL RE DOOL	FLAT SHEETS)	SHALL BE IN THE MIDDLE OF TH RTED AND MAINTAINED IN THIS		
10	PLACEMENT O	F CONCRETE.	0/FR 97551		יית שאוא עאוא עאיא שיייגע אוואנע אווא דעווא דער אוואנע דער אוואנע דער אוואנע דער אוואנע אווא דער אוואנע דער א איז ער דער אוואנע דער אוואנער אווא		ING PCr
10.		OVER AT LEAS	T THREE SUPP		LD TO EACH SUPPORT AT MININ RUCTION LOADS LIMIT DEELEN	IUM 12" INTERV	LOr ALS 1 D⊑
11.	TO 1/240 OF SF	AN WHEN LOA	DED WITH THE	WET WEIGHT	OF CONCRETE AND CONSTRUC A MINIMUM OF 6 x 6-W1 4 x W1 4	TION LOADS.	
12.	FABRIC REINFO	ORCING IN FLA	T SHEETS U.N. R FLOOR TOPP	O. INGS 2" OR MC	RE, PROVIDE 6 x 6-W1.4 x W1.4 v	WELDED WIRE	
13.	FABRIC REINFO	ORCING IN FLA R TREADS WITH	T SHEETS U.N. H CONCRETE F	.O. FILL, PROVIDE 4	x4-W1.4xW1.4 W.W.F. IN FLAT SI	HEETS.	
14.	BEND ALL HOR 2'-0" LAP.	RIZONTAL WALL	AND FOOTING	G BARS 1'-0" AR	OUND CORNERS OR PROVIDE (CORNER BARS V	VITH
15.	PROVIDE FOUN	NDATION DOWE	ELS FOR ALL W	/ALLS, PIERS, A	ND COLUMNS SAME SIZE AND S	SPACING AS	
16.	HORIZONTAL F BEAMS, ETC.) \$	LOORS WHICH	I HAVE DEFLEC SHED LEVEL. T	TING STRUCT	JRAL MEMBERS (STEEL JOISTS, KNESS NOTED IS MINIMUM. ADD	, UNSHORED ST O CONCRETE AS	EEL S
	NECESSARY TO	O OVERCOME I	MEMBER DEFL	ECTIONS. SHO	RED CONSTRUCTION SHALL BE	FINISHED TO A	١
17.	WHERE POCKE	ETS OR VOIDS /	ARE FORMED I	NTO CONCRET	E WALL FOR STEEL BEAMS OR	COLUMNS,	
18.	SPLICES:				OF THE LARGER BAR		
		TENSION SPLIC	CES IN ACCORE	DANCE WITH T			
	B. LAP ALL				HE FOLLOWING TABLE. MODIFY	LENGTHS AS N	
	B. LAP ALL	CONCRETE	COMPRESSIVE	E STRENGTH	1. INCREASE SPLICE LENGTH	H BY THE FOLLC) WIN
	B. LAP ALL	CONCRETE 3,000 PSI	COMPRESSIVE 4,000 PSI	E STRENGTH 5,000 PSI	1. INCREASE SPLICE LENGT	H BY THE FOLLC	
	B. LAP ALL BAR SIZE #3	CONCRETE 3,000 PSI 21"	COMPRESSIVE 4,000 PSI 19"	STRENGTH 5,000 PSI 17"	 HE FOLLOWING TABLE. MODIFY 1. INCREASE SPLICE LENGTH 2. NOTE: INCREASED LENGT 1. HORIZONTAL TOP BARS THAN 12" OF CONCRETE 	HENGTHS AS N H BY THE FOLLC HS ARE ACCUM WITH GREATER BELOW	
	B. LAP ALL BAR SIZE #3 #4	CONCRETE 3,000 PSI 21" 29"	COMPRESSIVE 4,000 PSI 19" 25"	5,000 PSI 17" 22"	 HE FOLLOWING TABLE. MODIFY 1. INCREASE SPLICE LENGTH 2. NOTE: INCREASED LENGT 1. HORIZONTAL TOP BARS THAN 12" OF CONCRETE 2. BAR SPACING LESS THA DIAMETERS 	HENGTHS AS N H BY THE FOLLC HS ARE ACCUM WITH GREATER BELOW N 2 BAR	WIN WIN +3 +5
	B. LAP ALL BAR SIZE #3 #4 #5	CONCRETE 3,000 PSI 21" 29" 36"	COMPRESSIVE 4,000 PSI 19" 25" 31"	5,000 PSI 17" 22" 28"	 HE FOLLOWING TABLE. MODIFY INCREASE SPLICE LENGTH NOTE: INCREASED LENGT HORIZONTAL TOP BARS THAN 12" OF CONCRETE BAR SPACING LESS THA DIAMETERS LIGHTWEIGHT CONCRET 	HENGTHS AS N H BY THE FOLLC HS ARE ACCUM WITH GREATER BELOW N 2 BAR	
	B. LAP ALL BAR SIZE #3 #4 #5 #6	CONCRETE 3,000 PSI 21" 29" 36" 43"	COMPRESSIVE 4,000 PSI 19" 25" 31" 37"	5,000 PSI 17" 22" 28" 33"	 INCREASE SPLICE LENGTH INCREASE SPLICE LENGTH NOTE: INCREASED LENGT HORIZONTAL TOP BARS THAN 12" OF CONCRETE BAR SPACING LESS THA DIAMETERS LIGHTWEIGHT CONCRET EPOXY-COATED TOP BA EPOXY-COATED OTHER 	HENGTHS AS N H BY THE FOLLC HS ARE ACCUM WITH GREATER BELOW N 2 BAR TE RS BARS	+3 +5 +3 +5 +3 +5 +3 +5
19.	B. LAP ALL BAR SIZE #3 #4 #5 #6 CONCRETE PR	CONCRETE 3,000 PSI 21" 29" 36" 43" OTECTION FOR	COMPRESSIVE 4,000 PSI 19" 25" 31" 37" R REINFORCEM	E STRENGTH 5,000 PSI 17" 22" 28" 33" MENT:	 HE FOLLOWING TABLE. MODIFY 1. INCREASE SPLICE LENGTH 2. NOTE: INCREASED LENGT 1. HORIZONTAL TOP BARS THAN 12" OF CONCRETE 2. BAR SPACING LESS THA DIAMETERS 3. LIGHTWEIGHT CONCRET 4. EPOXY-COATED TOP BA 5. EPOXY-COATED OTHER 	HENGTHS AS N H BY THE FOLLC HS ARE ACCUM WITH GREATER BELOW N 2 BAR TE RS BARS COVER	+3 +5 +3 +5 +3 +5
19.	B. LAP ALL BAR SIZE #3 #4 #5 #6 CONCRETE PR A. CONCRETE B. CONCRETE	CONCRETE 3,000 PSI 21" 29" 36" 43" OTECTION FOR CAST AGAINS EXPOSED TO	COMPRESSIVE 4,000 PSI 19" 25" 31" 37" R REINFORCEM T AND PERMAN EARTH OR WE	E STRENGTH 5,000 PSI 17" 22" 28" 33" MENT: NENTLY EXPOS ATHER	 INCREASE SPLICE LENGTH INCREASE SPLICE LENGTH NOTE: INCREASED LENGT HORIZONTAL TOP BARS THAN 12" OF CONCRETE BAR SPACING LESS THA DIAMETERS LIGHTWEIGHT CONCRET EPOXY-COATED TOP BA EPOXY-COATED OTHER 	HENGTHS AS N H BY THE FOLLC HS ARE ACCUM WITH GREATER BELOW N 2 BAR E RS BARS COVER 3"	
19.	B. LAP ALL BAR SIZE #3 #4 #5 #6 CONCRETE PR A. CONCRETE PR B. CONCRETE B. CONCRETE NO. 6 NO. 5	CONCRETE 3,000 PSI 21" 29" 36" 43" OTECTION FOR CAST AGAINS EXPOSED TO THROUGH NO. BAR, W31 OR D	COMPRESSIVE 4,000 PSI 19" 25" 31" 37" R REINFORCEM T AND PERMAN EARTH OR WE 18 BARS 31 WIRE AND S	E STRENGTH 5,000 PSI 17" 22" 28" 33" MENT: NENTLY EXPOS ATHER SMALLER	 HE FOLLOWING TABLE. MODIFY 1. INCREASE SPLICE LENGTH 2. NOTE: INCREASED LENGT 1. HORIZONTAL TOP BARS THAN 12" OF CONCRETE 2. BAR SPACING LESS THA DIAMETERS 3. LIGHTWEIGHT CONCRET 4. EPOXY-COATED TOP BA 5. EPOXY-COATED OTHER 	LENGTHS AS N H BY THE FOLLC HS ARE ACCUM WITH GREATER BELOW N 2 BAR TE RS BARS COVER 3" 2" 2" 1 1/2"	
19.	B. LAP ALL BAR SIZE #3 #4 #5 #6 CONCRETE PR A. CONCRETE B. CONCRETE B. CONCRETE NO. 6 NO. 5 C. CONCRETE SLABS	CONCRETE 3,000 PSI 21" 29" 36" 43" OTECTION FOR CAST AGAINS EXPOSED TO THROUGH NO. BAR, W31 OR D NOT EXPOSEI S AND WALLS	COMPRESSIVE 4,000 PSI 19" 25" 31" 37" R REINFORCEM T AND PERMAN EARTH OR WE 18 BARS 131 WIRE AND S D TO WEATHER	E STRENGTH 5,000 PSI 17" 22" 28" 33" MENT: NENTLY EXPOS ATHER SMALLER R OR IN CONTA	 INCREASE SPLICE LENGTH INCREASE SPLICE LENGTH NOTE: INCREASED LENGT HORIZONTAL TOP BARS THAN 12" OF CONCRETE BAR SPACING LESS THA DIAMETERS LIGHTWEIGHT CONCRET EPOXY-COATED TOP BA EPOXY-COATED OTHER SED TO EARTH 	HENGTHS AS N H BY THE FOLLC HS ARE ACCUM WITH GREATER BELOW N 2 BAR TE RS BARS COVER 3" 2" 2" 1 1/2"	+3 +5 +3 +3 +3 +5
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19.	B. LAP ALL BAR SIZE #3 #4 #5 #6 CONCRETE PR A. CONCRETE B. CONCRETE B. CONCRETE NO. 6 NO. 5 C. CONCRETE SLABS	CONCRETE 3,000 PSI 21" 29" 36" 43" COTECTION FOR CAST AGAINS EXPOSED TO THROUGH NO. BAR, W31 OR D NOT EXPOSEI S AND WALLS NO. 11 BAR AI	COMPRESSIVE 4,000 PSI 19" 25" 31" 37" R REINFORCEM T AND PERMAN EARTH OR WE 18 BARS 131 WIRE AND SI D TO WEATHER ND SMALLER .	E STRENGTH 5,000 PSI 17" 22" 28" 33" MENT: NENTLY EXPOS ATHER SMALLER R OR IN CONTA	 HE FOLLOWING TABLE. MODIFY 1. INCREASE SPLICE LENGTH 2. NOTE: INCREASED LENGT 1. HORIZONTAL TOP BARS THAN 12" OF CONCRETE 2. BAR SPACING LESS THA DIAMETERS 3. LIGHTWEIGHT CONCRET 4. EPOXY-COATED TOP BA 5. EPOXY-COATED OTHER 	LENGTHS AS N H BY THE FOLLC HS ARE ACCUM WITH GREATER BELOW N 2 BAR TE RS BARS COVER 2" 2" 2" 1 1/2" 3/4"	+3 +5 +3 +3 +3 +3 +3 +5
19. DF, FLC	B. LAP ALL BAR SIZE #3 #4 #5 #6 CONCRETE PR A. CONCRETE B. CONCRETE B. CONCRETE B. CONCRETE NO. 6 NO. 5 C. CONCRETE SLABS	CONCRETE 3,000 PSI 21" 29" 36" 43" OTECTION FOR CAST AGAINS EXPOSED TO THROUGH NO. BAR, W31 OR D NOT EXPOSEI S AND WALLS NO. 11 BAR AI OPENINGS	COMPRESSIVE 4,000 PSI 19" 25" 31" 37" R REINFORCEM T AND PERMAN EARTH OR WE 18 BARS 131 WIRE AND SI 10 TO WEATHER ND SMALLER .	E STRENGTH 5,000 PSI 17" 22" 28" 33" MENT: NENTLY EXPOS ATHER SMALLER R OR IN CONTA	 HE FOLLOWING TABLE. MODIFY 1. INCREASE SPLICE LENGTH 2. NOTE: INCREASED LENGT 1. HORIZONTAL TOP BARS THAN 12" OF CONCRETE 2. BAR SPACING LESS THA DIAMETERS 3. LIGHTWEIGHT CONCRET 4. EPOXY-COATED TOP BA 5. EPOXY-COATED OTHER 	LENGTHS AS N H BY THE FOLLC HS ARE ACCUM WITH GREATER BELOW N 2 BAR TE RS BARS COVER 2" 2" 2" 11/2" 3/4"	+3 +5 +3 +3 +3 +3 +3 +5
19. DF, FLO THE C SLEE	B. LAP ALL BAR SIZE #3 #4 #5 #6 CONCRETE PR A. CONCRETE B. CONCRETE B. CONCRETE B. CONCRETE SLABS DOR, OR WALL CONTRACTOR S VES AND OPEN	CONCRETE 3,000 PSI 21" 29" 36" 43" OTECTION FOF CAST AGAINS EXPOSED TO THROUGH NO. BAR, W31 OR D NOT EXPOSEI S AND WALLS NO. 11 BAR AI OPENINGS SHALL VERIFY / INGS REQUIRE	COMPRESSIVE 4,000 PSI 19" 25" 31" 37" R REINFORCEM T AND PERMAN EARTH OR WE 18 BARS 131 WIRE AND SI 14 WIRE AND SI 15 TO WEATHER ND SMALLER .	E STRENGTH 5,000 PSI 17" 22" 28" 33" MENT: NENTLY EXPOS ATHER SMALLER COR IN CONTA R OR IN CONTA	 HE FOLLOWING TABLE. MODIFY 1. INCREASE SPLICE LENGTH 2. NOTE: INCREASED LENGT 1. HORIZONTAL TOP BARS THAN 12" OF CONCRETE 2. BAR SPACING LESS THA DIAMETERS 3. LIGHTWEIGHT CONCRET 4. EPOXY-COATED TOP BA 5. EPOXY-COATED OTHER SED TO EARTH CT WITH GROUND ER, SIZE, AND LOCATION OF AL CTRICAL ITEMS.	LENGTHS AS N H BY THE FOLLC HS ARE ACCUM WITH GREATER BELOW N 2 BAR TE RS BARS COVER 2" 2" 2" 1 1/2" 3/4"	+3 +5 +3 +3 +3 +3 +3 +5
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19. THE (SLEE SLEE INTEC NO S	B. LAP ALL BAR SIZE #3 #4 #5 #6 CONCRETE PR A. CONCRETE B. CONCRETE B. CONCRETE B. CONCRETE NO. 6 NO. 5 C. CONCRETE SLABS OOR, OR WALL CONTRACTOR S VES AND OPEN VES AND OPEN SRITY OF THE F TRUCTURAL EL	CONCRETE 3,000 PSI 21" 29" 36" 43" OTECTION FOF CAST AGAINS EXPOSED TO THROUGH NO. BAR, W31 OR D NOT EXPOSEI S AND WALLS NO. 11 BAR AI OPENINGS SHALL VERIFY A INGS REQUIRE INGS SHALL BE ROOF, FLOOR, A EMENTS ARE T	COMPRESSIVE 4,000 PSI 19" 25" 31" 37" R REINFORCEM T AND PERMAN EARTH OR WE 18 BARS 31 WIRE AND SI 10 TO WEATHER ND SMALLER . AND COORDINA D FOR MECHA E LOCATED IN A AND WALL SYS TO BE CUT UNL	E STRENGTH 5,000 PSI 17" 22" 28" 33" MENT: NENTLY EXPOS ATHER SMALLER COR IN CONTA SMALLER COR IN CONTA ATE THE NUMB NICAL OR ELEG A MANNER THA STEMS. LESS SPECIFIC.	 HE FOLLOWING TABLE. MODIFY 1. INCREASE SPLICE LENGTH 2. NOTE: INCREASED LENGT 1. HORIZONTAL TOP BARS THAN 12" OF CONCRETE 2. BAR SPACING LESS THA DIAMETERS 3. LIGHTWEIGHT CONCRET 4. EPOXY-COATED TOP BA 5. EPOXY-COATED OTHER SED TO EARTH SED TO EARTH CT WITH GROUND ER, SIZE, AND LOCATION OF AL CTRICAL ITEMS. T WILL MAINTAIN THE STRUCTURALLY APPROVED BY THE ENGIN	LENGTHS AS N H BY THE FOLLC HS ARE ACCUM WITH GREATER BELOW N 2 BAR 'E RS BARS COVER 2" 2" 1 1/2" 3/4" L JRAL EER.	
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	WALL THICKNESS			
BAR SIZE	8" C.M.U.	12" C.M.U.		
#3	18"	18"		
#4	25"	25"		
#5	31"	31"		
#6	57"	52"		
#7	79"	61"		
#8	112"	75"		

14. SEE DETAILS AND SCHEDULES FOR LOCATIONS AND SIZES OF HORIZONTAL AND VERTICAL REINFORCEMENT. 15. REINFORCE BOND BEAMS WITH (2) #5 CONTINUOUS, UNLESS OTHERWISE NOTED. PROVIDE CORNER BARS

FOR ALL BOND BEAM REINFORCEMENT. 16. IN ADDITION TO SPACING INDICATED IN SCHEDULE, PROVIDE VERTICAL BARS AT ALL CORNERS, ENDS, JAMBS,

INTERSECTIONS AND BOTH SIDES OF CONTROL JOINTS. 17. EXTEND ALL VERTICAL REINFORCEMENT THRU MID-HEIGHT BOND BEAMS. EXTEND VERTICAL REINFORCING INTO

BOND BEAMS AT TOP OF WALL AND TERMINATE AT 2" DOWN FROM TOP OF WALL. 18. PROVIDE DOWELS FROM SUPPORTING MEMBER (FOOTING, BEAM, OR SLAB) FOR ALL REINFORCED WALLS, SAME SIZE, LOCATION, AND SPACING AS WALL REINFORCING.

19. VERTICAL REINFORCEMENT SHALL BE CENTERED IN CELLS OF MASONRY UNIT. UNLESS OTHERWISE NOTED. 20. BAR POSITIONERS SHALL BE USED TO HOLD VERTICAL AND BOND BEAM REINFORCEMENT IN PROPER ALIGNMENT.

21. VERTICAL BARS SHALL BE HELD IN POSITION AT TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 4 FEET.

22. GROUTING OF MASONRY LINTELS OVER OPENINGS SHALL BE ACCOMPLISHED IN ONE CONTINUOUS OPERATION. 23. WHERE LOW CUT WEB, OPEN CELLED C.M.U. ARE USED FOR BOND BEAMS, PROVIDE A CONTINUOUS METAL LATH GROUT RETAINER IN THE BED JOINT TO RETAIN GROUT IN CELLS.

	GROUT TYPE	MAXIMUM GROUT POUR HEIGHT	MINIMUM WIDTH OF GROUT SPACE	MINIMUM GROUT SPACE DIMENSIONS FOR GROUTING CELLS OF HOLLOW UNITS
	FINE	1'	3/4"	1 1/2" x 2"
	FINE	5'	2"	2" x 3"
	FINE	12'	2 1/2"	2 1/2" x 3"
	FINE	24'	3"	3" x 3"
	COARSE	1'	1 1/2"	1 1/2" x 3"
	COARSE	5'	2"	2 1/2" x 3"
	COARSE	12'	2 1/2"	3" x 3"
	COARSE	24'	3"	3" x 4"
A. B. C. D. 34. SE 35. UN 36. AT 37. DC 3/4 AT 37. DC 3/4 38. MI PE WI SH AT 39. SE GF FU 40. VE TE 41. HC	AT STRUCTURA REINFORCING B AT SOLID GROUT CLEANOUT CLOS GROUT POURS S BEHIND TRIM, CI SHALL BE APPLI E ARCHITECTURA ILESS OTHERWIS VERTICAL CONT SCONTINUOUS. F CONTROL JOINT ONOT CONSTRUC CONTROL JOINT CONTROL SCONT ALL BE HOOKED TO O.C. ARE REC COUT ONLY CELLS COUT ONLY CELLS COUT ONLY CELLS COUT ONLY CELLS COUT ONLY CELLS	LLY REINFORCED WAL AR. TED MASONRY, PROVII SURES SHALL BE BRAG SHALL BE PLANNED SC EILING, OR OTHER FIN ED IN POURS LESS TH AL DRAWINGS FOR LO E SHOWN OR NOTED, ROL JOINTS, BOND BE PROVIDE TWO 3/4" DIAI EAM. GREASE ONE ENI TO PREVENT BINDING TO PREVENT BINDING TO NON-LOADBEARING TRUCTURE AND INFILI EQUIREMENTS. TAL REINFORCING FOF IG WALLS, LAID IN RUN T 48" O.C. MAXIMUM VE 180° AROUND THE VEF QUIRED BETWEEN BON FOR VERTICAL REINFO S WHICH CONTAIN REI WALL FOR SIMPLICITY CING BARS IN LOAD-B STANDARD HOOK ABC ORCING SHALL BE PRI	LS PROVIDE CLEANOU DE CLEANOUT HOLES A CED TO RESIST GROUT D THAT CLEANOUT HOL ISHES. WHERE CLEAN AN 5 FEET TALL TO FOI CATIONS OF VERTICAL SPACING OF CONTROL AM REINFORCEMENT A METER SMOOTH DOWE D. PROVIDE 3/8" FOAM I G. MASONRY TIGHT TO U WITH COMPRESSIBLE R CONCRETE MASONRY INING BOND, SHALL BE ERTICAL SPACING. BOT RTICAL REINFORCING EN D BEAMS. DRCING SIZE AND SPAC NFORCING SIZE AND SPAC NFORCING STEEL AND , DUE TO EXCESSIVE W EARING WALLS, SHEAF DVE THE BOND BEAM R OVIDED WITHIN 16" OF	T HOLES AT EACH STRUCTURAL VERTICA AT NOT MORE THAN 32 INCHES ON CENTE PRESSURES. ES ARE CONCEALED BELOW SLAB OR OUTS CANNOT BE CONCEALED, GROUT RGO CLEANOUTS. CONTROL JOINTS. JOINTS SHALL NOT EXCEED 25 FEET. AND JOINT REINFORCEMENT SHALL BE ELS BY 1'-4" ACROSS EACH CONTROL JOIN POUR STOP IN HEAD JOINT OF ALL BOND NDERSIDE OF STRUCTURE. PROVIDE MIN INDERSIDE TO VELONT WIT, GALVANIZED STEEL WIRE CING. FOR WALLS LAID IN RUNNING BOND. BOND BEAMS. IT IS NOT PERMISSIBLE TO VELONT AND PERIMETER WALLS SHALL VELONT AND PERIMETER WALLS SHALL VENTOR AND PERIMETER WALLS SHALL VENTOR AND BOTTOM OF ALL C.M.U. WA
STEEL	CONSTRUCTION			

1.	011	LE DE FAILING, FABRICATION, AND ERECTION SHALL
	CO	DE OF STANDARD PRACTICE, AND THE AWS STRUCT
2.	CO	NNECTIONS - WELDED OR HIGH STRENGTH BOLTED:
	В.	A325-N WITH HARDENED WASHERS - USE FOR ALL (
		AND OTHER CONNECTIONS AS NOTED ON DRAWING
	C.	UNLESS SNUG TIGHT CONNECTIONS ARE NOTED OF
		CONNECTIONS.
		SHALL BE TIGHTENED TO FULL PRETENSIONING LO
	D.	UNLESS SPECIFICALLY NOTED ON THE DRAWINGS (
		ENGINEER, ALL BOLTS FOR THE PROJECT SHALL BE
	Ε.	USE STANDARD HOLES WITH THE FOLLOWING EXCE
		BOLTS ARE LOADED IN TENSION; SHORT SLOTTED I
		PERPENDICULAR TO THE SLOT.
	F.	HARDENED WASHERS SHALL BE USED OVER ALL OV
		OUTER PLY. WHERE LONG-SLOTTED HOLES ARE US
		WASHERS OR CONTINUOUS BAR WITH STANDARD F
	G.	WHERE REACTION IS NOTED, DEVELOP SAME. WHE
		CONNECTIONS SHALL DEVELOP ONE-HALF OF THE 1
		FOR COMPOSITE BEAMS, SEE TABLE.
	Н.	WHEREVER POSSIBLE, USE FRAMED BEAM CONNEC
		10-4 OF THE AISC STEEL CONSTRUCTION MANUAL, 13
		ANGLES AND PLATES SHALL BE NOT LESS THAN ON
	J.	PREAPPROVED CONNECTION DETAILS ARE PROVIDE
	K.	SINGLE PLATE SHEAR CONNECTIONS ARE NOT PER
		AT FIELD APPLIED CONNECTIONS, OR CONNECTIONS
		CONNECTIONS, MOMENT CONNECTIONS, PIPE COLL
		OR LESS, OR CONNECTIONS WITH REACTIONS LESS
	L.	THROUGH PLATE CONNECTIONS AT TUBE COLUMNS
		SHEAR CONNECTIONS TO TUBE COLUMNS SHALL BE
		EVALET AN MOTER ARAL

EXCEPT AS NOTED ABOVE. 3. WELDING ELECTRODES SHALL BE E70XX EXCEPT WHERE OTHER ELECTRODES ARE REQUIRED FOR COMPATIBILITY WITH MATERIAL BEING WELDED. 4. SHOP DRAWINGS ARE REQUIRED AND SHALL NOTE TYPE OF ELECTRODES, SIZE OF ALL WELDS, AND TYPE AND SIZE OF ALL BOLTS. 5. SEE SPECIFICATIONS FOR ALL PAINTING REQUIREMENTS. BEAMS BEARING ON MASONRY SHALL BEAR A MINIMUM OF 8" ONTO THE WALL, UNLESS OTHERWISE NOTED. BEAR BEAMS FULL LENGTH OF BEARING PLATES. MASONRY SHALL BE BUILT TIGHT AROUND BEAM UNLESS OTHERWISE NOTED. 7. ALL SHOP AND FIELD WELDING SHALL BE DONE BY A CERTIFIED WELDER. 8. FOR CONNECTIONS TO EXISTING CONCRETE, LOCATE THE REINFORCING BY MEANS OF A REBAR DETECTOR PRIOR TO DRILLING. ADJUST THE CONNECTION AS REQUIRED TO AVOID CUTTING ANY REINFORCING. DO NOT WELD TO EXISTING STEEL WITHOUT WRITTEN APPROVAL FROM THE ENGINEER. 10. MISCELLANEOUS HANGING LOADS SUCH AS STAIR STRINGERS, PIPES, MECHANICAL UNITS, ETC., SUPPORTED BY STEEL MEMBERS SHALL HAVE THESE LOADS APPLIED IN SUCH A MANNER THAT NO TORSIONAL FORCES

ARE INDUCED IN THESE MEMBERS, I.E., LOADS SHALL PASS THROUGH THE CENTERLINE OF WIDE FLANGE SECTIONS AND THROUGH THE SHEAR CENTER OF CHANNELS. STEEL DECK CONSTRUCTION 1. STEEL DECK DETAILING, FABRICATION, AND ERECTION SHALL CONFORM TO THE LATEST, AWS STRUCTURAL WELDING CODE AND THE STEEL DECK INSTITUTE SPECIFICATIONS. 2. STEEL ROOF DECK SHALL BE CONTINUOUS OVER A MINIMUM OF 3 SPANS. STEEL FLOOR DECK SHALL BE CONTINUOUS OVER A MINIMUM OF 2 SPANS. 3. DO NOT HANG OR SUPPORT ANY LOADS SUCH AS STUD WALLS, BULKHEADS, PIPES, ETC FROM STEEL ROOF DECK. 4. WHERE JOISTS/BEAMS DO NOT ALIGN ON OPPOSITE SIDES OF A GIRDER AND A JOINT IS MADE IN THE DECK, PROVIDE AN ADDITIONAL LAYER OF SINGLE SPAN DECK ABOVE CANTILEVERED ENDS TO SPAN BETWEEN SUPPORTS. WOOD CONSTRUCTION 1. WOOD STRESS GRADE FOR ALL STRUCTURAL FRAMING MEMBERS: SPRUCE PINE FIR, STUD GRADE 2x4 = SPRUCE PINE FIR, NO. 2 GRADE 2x6 =2x8, 2x10, 2x12 = SOUTHERN PINE, NO. 2 GRADE . CONSTRUCTION SHALL CONFORM TO THE NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION. . PROVIDE HEADERS OVER ALL OPENINGS IN NONBEARING 2x4 WALLS. ADD ADDITIONAL MEMBER FOR 2x6 WALLS MEMBER SIZE CLEAR SPA) TO 5'-0" (2) 2 x 6 5'-0" TO 7'-0" (2) 2 x 8 7'-0" TO 9'-0" (2) 2 x 10 9'-0" TO 11'-0" (2) 2 x 12 4. CUTTING OF JOISTS AND BEAMS FOR PIPES SHALL NOT BE PERMITTED WITHOUT THE APPROVAL OF THE ENGINEER. 5. BOLT HOLES IN WOOD SHALL BE 1/16" OVERSIZE. WASHERS SHALL BE USED ON ALL BEARINGS OF HEADS AND NUTS AGAINST WOOD. WASHERS SHALL BE STANDARD PLAIN WASHERS EXCEPT AS OTHERWISE NOTED BOLTS SHALL CONFORM TO ASTM A307. BOLTS, NUTS, AND WASHERS SHALL BE GALVANIZED WHERE EXPOSED TO THE WEATHER. STUD WALLS SUPPORTING BEAMS SHALL HAVE POSTS UNDER BEARING UNLESS OTHERWISE NOTED. 7. ALL POSTS OR STUD WALLS INTERSECTING WITH CONCRETE OR MASONRY WALLS SHALL HAVE THE END STUD OR POST BOLTED WITH 1/2" DIAMETER SLEEVE ANCHORS AT 4'-0" O.C. JOIST HANGERS, SHEET METAL FRAMING CLIPS AND ANGLES, STRAPS, ETC., SHALL BE AS MANUFACTURED BY THE SIMPSON COMPANY OR OTHER APPROVED. METAL CONNECTORS SHALL BE GALVANIZED TO G90 THICKNESS FOR UNTREATED LUMBER AND TO G185 THICKNESS FOR PRESERVATIVE TREATED LUMBER. 9. INSTALL FLOOR JOISTS WITH CROWN EDGE UP.

10. INSTALL SOLID BLOCKING @ 6'-0" O.C. MAX IN FIRST THREE FLOOR JOIST SPACES WHERE FLOOR JOISTS SPAN PARALLEL TO WALL. CONNECT BLOCKING TO WALL WITH METAL FRAMING ANGLES. 11. UNDER NON-LOAD BEARING PARTITIONS PARALLEL TO JOISTS, INSTALL DOUBLE JOISTS SEPARATED BY SOLID BLOCKING EQUAL TO WIDTH OF STUDS ABOVE. 12. INSTALL BRIDGING OF TYPE INDICATED BELOW BETWEEN JOISTS WHERE NOMINAL DEPTH-TO-THICKNESS RATIO EXCEEDS 6, AT INTERVALS OF 8 FEET. A. DIAGONAL WOOD BRIDGING FORMED FROM BEVEL CUT NOMINAL 1-INCH BY 4-INCH LUMBER, DOUBLE-CROSSED AND NAILED BOTH ENDS TO JOISTS. B. SOLID WOOD BRIDGING 2 INCHES THICK BY DEPTH OF JOIST, END NAILED TO JOIST. C. STEEL BRIDGING INSTALLED TO COMPLY WITH BRIDGING MANUFACTURER'S DIRECTIONS.

EQUIPMENT SUPPORT

1. WHERE EQUIPMENT IS SUPPORTED FROM WOOD FRAMING (INCLUDING RAFTERS, JOISTS, ETC), ATTACHMENT SHALL BE MADE SUCH THAT LOAD IS BEARING ON WOOD FRAMING. NO TENSION CONNECTION SHALL BE PERMITTED. 2. AT POINT OF LOAD, INSTALL 2x BLOCKING IN DIRECTION PERPENDICULAR TO SPAN OF LOADED MEMBER. 3. APPLY LOADS TO MEMBERS THAT ARE DESIGNATED FOR ADDITIONAL LOADS AS NOTED ON DRAWINGS. NOTIFY ENGINEER IF LOADS NOTED ARE EXCEEDED, OR IF LOADS DO NOT OCCUR IN LOCATION ANTICIPATED AND AS INDICATED ON FRAMING PLANS.

24. VERTICAL REINFORCING BARS SHALL HAVE A MINIMUM CLEARANCE OF 3/4" FROM THE MASONRY SURFACE AND NOT LESS THAN ONE BAR DIAMETER BETWEEN BARS. 25. MAINTAIN CLEAR DISTANCE OF 1/4" MINIMUM FOR FINE GROUT OR 1/2" MINIMUM FOR COARSE GROUT

BETWEEN REINFORCING BARS AND ANY FACE OF MASONRY UNIT. 26. WHERE MULTIPLE FULL HEIGHT BARS ARE SPECIFIED FOR THE SAME MASONRY CELL, THE CLEAR DISTANCE BETWEEN PARALLEL BARS SHALL NOT BE LESS THAN THE NOMINAL BAR DIAMETER, NOR 1 INCH. 27. REMOVE MORTAR PROTRUSIONS GREATER THAN 1/2" FROM CELLS BEFORE GROUTING.

INT AT) BEAMS NIMUM) MEET

1ND RCED CING BAR

ALLS

1 STEEL DETAILING. FABRICATION. AND ERECTION SHALL CONFORM TO THE AISC SPECIFICATIONS AND FURAL WELDING CODE. CONNECTIONS OTHER THAN SLIP CRITICAL

N THE DRAWINGS AS BEING PERMITTED, ALL BOLTS

OR WITHOUT WRITTEN PERMISSION FROM THE E OF ONE ASTM TYPE AND ONE DIAMETER. EPTIONS: OVERSIZE HOLES ARE PERMITTED WHEN HOLES ARE PERMITTED FOR SHEAR LOADING VERSIZED OR SHORT-SLOTTED HOLES IN AN SED IN AN OUTER PLY, 5/16" THICK A36 PLATE

HOLES SHALL BE PROVIDED. ERE NOT NOTED, FOR NON-COMPOSITE BEAMS, TOTAL UNIFORM LOAD CAPACITY OF THE BEAM; TIONS AS LISTED IN TABLES 10-1, 10-2, 10-3 OR 3TH EDITION. THE LENGTH OF CONNECTION

E-HALF OF THE T DISTANCE OF THE BEAM WEB. ED ON DRAWING MITTED WHERE THE REACTION EXCEEDS 50 KIPS, NS TO COLUMNS (OTHER THAN AT SKEWED JMNS, TUBE COLUMNS WITH FACE DIMENSION 4" S THAN 15 KIPS). SARE NOT PERMITTED, UNLESS NOTED OTHERWISE. E WT OR DOUBLE ANGLE KNIFE CONNECTIONS

SPECIAL INSPECTION

- 1. SPECIAL INSPECTIONS AS DEFINED IN SECTION 1704.0 OF THE KENTUCKY BUILDING CODE ARE REQUIRED. 2. SPECIAL INSPECTIONS SHALL BE PERFORMED BY A QUALIFIED TESTING AGENCY APPROVED BY THE ARCHITECT AND THE STRUCTURAL ENGINEER AND PAID FOR BY THE OWNER.
- 3. THE INSPECTOR SHALL OBSERVE WORK FOR CONFORMANCE WITH THE APPROVED STRUCTURAL DRAWINGS AND SPECIFICATIONS AND PREPARE INSPECTION REPORTS STATING HIS/HER OBSERVATIONS. COPIES OF THE INSPECTION REPORTS SHALL BE SUBMITTED TO THE CONTRACTOR, THE ARCHITECT AND
- THE STRUCTURAL ENGINEER. 4. ALL DISCREPANCIES BETWEEN THE CONSTRUCTION DOCUMENTS AND THE WORK BEING PERFORMED SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THE
- DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF ARCHITECT AND THE STRUCTURAL ENGINEER PRIOR TO THE COMPLETION OF THAT PHASE OF THE WORK.
- 5. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL REPORT OF INSPECTIONS DOCUMENTING COMPLETION OF ALL REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS. 6. SPECIAL INSPECTIONS ARE REQUIRED FOR THE FOLLOWING WORK:
 - INSPECTION OF FABRICATORS PERFORM SPECIAL INSPECTIONS PER SECTION 1704.2 OF THE KENTUCKY BUILDING CODE. STEEL CONSTRUCTION PERFORM SPECIAL INSPECTIONS PER SECTION 1704.3 OF THE KENTUCKY BUILDING CODE. CONCRETE CONSTRUCTION PERFORM SPECIAL INSPECTIONS PER SECTION 1704.4 OF THE KENTUCKY BUILDING CODE. MASONRY CONSTRUCTION PERFORM SPECIAL INSPECTIONS PER SECTION 1704.5 OF THE KENTUCKY BUILDING CODE.

WOOD CONSTRUCTION PERFORM SPECIAL INSPECTIONS PER SECTION 1704.6 OF THE KENTUCKY BUILDING CODE.

BASIS OF PAYMENT

- ALL REPAIRS SHALL BE BID (LUMP SUM) BASED ON QUANTITIES SHOWN IN THE DRAWINGS. FINAL PAYMENT WILL BE ADJUSTED FROM BASE BID, POSITIVE OR NEGATIVE, BASED ON THE UNIT COST AND ACTUAL FIELD VERIFIED QUANTITIES.
- UNIT COSTS SHALL BE THE SAME ADDITIVE OR DEDUCTIVE COST TO THE BASE BID. A PRE CONSTRUCTION MEETING SHALL BE HELD ON SITE AND CONDUCTED BY THE GENERAL CONTRACTOR. THE OWNER (UNIVERSITY OF KENTUCKY), THE USER (UK ART SCHOOL), AND THE ENGINEER SHALL ATTEND.



A	AMERICAN PLYWOOD ASSOCIATION
СН	ARCHITECTURAL
Т	BOTTOM
R	CLEAR
NT	CANTILEVER
<i>I</i> .U.	CONCRETE MASONRY UNIT
NT	CONTINUOUS
	DEEP
т	
IGS	DRAWINGS
	FACH FACE
=v	ELEVATION
V.	EACH WAY
).S.	EDGE OF SLAB
-	EXISTING
Р	EXPANSION
.E.	FINISHED FLOOR ELEVATION
.C.	FIBER REINFORCED CONCRETE
.Т.	FIRE RETARDANT TREATED
	FIELD VERIFY
-	GAUGE
LV	GALVANIZED
RIZ	HORIZONTAL
S	HOLLOW STRUCTURAL SECTION
-	

HSS

LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LFH	LONG FACE HORIZONTAL
LFV	LONG FACE VERTICAL
LVL	LAMINATED VENEER LUMBER
MAX	MAXIMUM
MIN	MINIMUM
N.T.S.	NOT TO SCALE
0.C.	ON CENTER
OPH	OPPOSITE HAND
P.A.F.	POWDER ACTUATED FASTENER
P.T.	PRESERVATIVE TREATED
PL	PLATE
R	RADIUS
SIM	SIMILAR
S.O.G.	SLAB ON GRADE
TYP	TYPICAL
U.N.O.	UNLESS NOTED OTHERWISE
VERT	VERTICAL
W	WIDE
W.W.F.	WELDED WIRE FABRIC

I.C.F. INSULATED CONCRETE FORM

MATERIAL LEGEND

ENGINEERED FILL



CONCRETE

PLAN NOTES

- 1. ELEVATIONS ARE SHOWN TO THE TOP OF FOUNDATION AND ARE REFERENCED FROM FINISHED FIRST FLOOR (LOW) ELEVATION 0'-0".
- 2. SEE THIS SHEET FOR GENERAL NOTES.
- 3. EXISTING FOUNDATIONS, WALLS, ETC. ARE SHOWN FOR REFERENCE ONLY. CONTRACTOR SHALL FIELD VERIFY DIMENSIONS RELEVANT TO HIS WORK.
- 4. SEE DWG S2.0 FOR REFERENCE PLAN INDICATING INTERIOR AND EXTERIOR WALL ELEVATIONS.

TAG NOTES

- $\langle 1 \rangle$ ex basement wall to be underpinned/shored prior TO EXCAVATION AND PLACEMENT OF NEW FOUNDATIONS.
- $\langle 2 \rangle$ EX COLUMN NOT ON GRID A OR B TO BE REMOVED ONCE FIRST FLOOR FRAMING IS COMPLETELY REPAIRED (OR SHORED).
- $\begin{pmatrix} 3 \end{pmatrix}$ EX CONC EQUIPMENT PADS TO BE REMOVED DOWN TO CONC SLAB LEVEL. ROUGHENED SURFACE TO BE CLEANED OF ANYTHING THAT WOULD PREVENT BOND TO TOPPING SLAB.
- $\langle 4 \rangle$ EX FOUNDATION TO BE REMOVED.

 \langle 5 ight
angle NEW CONC RAMP PER DET __/S__

FOUNDATION LEGEND

⊥EX F.F.E. =	(+14'-0")
•	= EXISTING FINISHED FLOOR SPOT ELEVATION REFERENCE FROM EX FINISHED FIRST (LOW) FLOOR ELEVATION 100'-
	= 8" C.M.U. WALL REINFORCED W/ #5@32" O.C. VERT. 8" BOND BEAM REINFORCED W/ (2) #5 SHALL BE PLACED AT FLOOR AND INTERMEDIATE LANDING LEVELS (SEE SECTIONS).
<u> </u>	= EXISTING MASONRY WALL.
	= 3" NOMINAL THICKNESS SELF LEVELING CONCRETE TOPPING. FINISHED FLOOR ELEVATION SHALL BE TO HIGHEST EXISTING BOTTOM OF BASE PLATE ELEVATION.



CONTRACTOR TO VERIFY ALL DIMENSIONS IN THE FIELD







- 1. ELEVATIONS ARE SHOWN TO THE TOP OF FOUNDATION OR TOP OF BEAM AND ARE REFERENCED FROM FINISHED
- FIRST FLOOR (LOW) ELEVATION 0'-0". 2. SEE DWG S1.0 FOR GENERAL NOTES.
- 3. EXISTING FOUNDATIONS, WALLS, FRAMING, ETC. ARE SHOWN FOR REFERENCE ONLY. CONTRACTOR SHALL FIELD VERIFY DIMENSIONS RELEVANT TO HIS WORK.
- 4. SEE DWG S2.0 FOR REFERENCE PLAN INDICATING INTERIOR AND EXTERIOR WALL ELEVATIONS.
- 5. ALL EXTERIOR STEEL FRAMING SHALL BE HOT DIPPED GALVANIZED. THIS INCLUDES STEEL LINTELS INSTALLED IN EXTERIOR WALLS.

TAG NOTES

- $\langle 1 \rangle$ INSTALL NEW WINDOWS/DOORS PER ARCH DWGS IN EXISTING INFILL LOCATIONS. SEE DWG S2.0 & WALL ELEVATIONS FOR ADDITIONAL INFORMATION.
- $\langle 2 \rangle$ INSTALL CONCRETE BASE AT MONUMENTAL STAIR.
- REMOVE EXISTING WALLS, FOUNDATIONS, BOILERS AND 3 MASONRY SUPPORT COMPONENTS, EQUIPMENT PADS, CONCRETE SLAB ON GRADE, TRENCHES ETC. INSTALL NEW SLAB ON GRADE ON 10 MIL POLYETHYLENE VAPOR RETARDER OVER 4" MINIMUM COMPACTED CRUSHED STONE OR DENSE GRADE AGGREGATE.
- NEW 6"± CONCRETE PADS FOR EQUIPMENT. 4 COORDINATE LENGTH x WIDTH OF PAD WITH EQUIPMENT REQUIREMENTS. SEE DETAIL H/S3.1.
- $\langle 5 \rangle$ PORTION OF EX MASONRY WALL AT ELECTRICAL ROOM
- TO REMAIN. $\langle 6 \rangle$ EX B1 WOOD BEAMS TO REMAIN.
- 7 > NEW 1'-0" THICK CONC FOOTING FOR MASONRY
- SUPPORT. (8) INSTALL (#) UNDERPINNING PILES TO STABILIZE EXTERIOR WALL FOUNDATION. (MINIMUM ALLOWABLE CAPACITY = 25k WITH 1/16" DISPLACEMENT).

I FGFND

S-3.0	= 2 7/16" NORMAL WEIGHT CONCRETE REINFORCED W/ 6x6-W2.1xW2.1 W.W.F. ON 9/16" 24 GA GALVANIZED AND THEN PRIME PAINTED NON-COMPOSITE STEEL FORM DECK (3" TOTAL THICKNESS) (+10'-0").
S-5.0	 = 2" LIGHT WEIGHT CONCRETE REINFORCED W/ 6x6-W2.1xW2.1 W.W.F. ON 3" 18 GA GALVANIZED AND THEN PRIME PAINTED COMPOSITE STEEL DECK (5" TOTAL THICKNESS).
EX DECK TYPE 1	= (3) LAYERS 1x DECKING (2 1/4" TOTAL THICKNESS).
TYPE 4A	 4x T&G WOOD DECKING IN COMBINATION SIMPLE & TWO-SPAN CONTINUOUS LAYUP. (SOUTHERN PINE DENSE STANDARD GRADE)
LVL	= (2) 1 3/4x11 1/4 MICROLAM 1.9E (OR APPROVED EQUA
C12	= C12x20.7
C8	= C8x11.5
C5	= C5x6.7
EX F.F.E. =	: (+14'-0") = EXISTING FINISHED FLOOR SPOT ELEVATION REFERE FROM EX FINISHED FIRST (LOW) FLOOR ELEVATION (
\$XX\$	= 8" C.M.U. WALL REINFORCED W/ #5@32" O.C. VERT. 8" BOND BEAM REINFORCED W/ (2) #5 SHALL BE PLAC AT FLOOR AND INTERMEDIATE LANDING LEVELS (SEE SECTIONS).
1 1	

> =	EXISTING MASONRY	WALL

WOOD BEAM SCHEDULE				
MARK	ACTUAL SIZE (WIDTH x DEPTH)			
B1	11x13			
B2	13x15			
B3	(5) 2x13 1/2			
NOTEO				







PLAN NOTES

- 1. ELEVATIONS ARE SHOWN TO THE TOP OF EX CONC F.F.E. AND ARE REFERENCED FROM FINISHED FIRST FLOOR (LOW) ELEVATION 0'-0".
- 2. SEE DWG S1.0 FOR GENERAL NOTES. 3. EXISTING FOUNDATIONS, WALLS, FRAMING, ETC. ARE
- SHOWN FOR REFERENCE ONLY. CONTRACTOR SHALL FIELD VERIFY DIMENSIONS RELEVANT TO HIS WORK.
- 4. SEE DWG S2.0 FOR REFERENCE PLAN INDICATING INTERIOR AND EXTERIOR WALL ELEVATIONS.
- 5. ALL EXTERIOR STEEL FRAMING SHALL BE HOT DIPPED GALVANIZED. THIS INCLUDES STEEL LINTELS INSTALLED IN EXTERIOR WALLS.

TAG NOTES

- 1
 EXISTING WOOD BEAM TO BE REMOVED.

 SEE DETAIL_____FOR ADDITIONAL INFORMATION.

 2 INFILL EXISTING WALL OPENING W/ 8" C.M.U. (SEE ARCH & ELEVATOR MANUFACTURER DRAWINGS FOR DOOR LOCATION). $\langle 3 \rangle$ INSTALL NEW WINDOWS PER ARCH DRAWINGS. SEE DRAWING S2.0 AND WALL ELEVATIONS FOR ADDITIONAL INFORMATION. $\langle 4 \rangle$ NEW DOOR OPENING PER ARCH DRAWINGS. SEE DRAWING S2.0 AND WALL ELEVATIONS FOR ADDITIONAL INFORMATION. $\langle 5 \rangle$ Shore existing masonry wall, then remove AND DISPOSE OF EXISTING ROOFING, DECKING AND STEEL BEAMS. 6 EXISTING EXTERIOR WALL TO BE ATTACHED TO EXISTING FLOOR DIAPHRAGM PER DETAIL $\langle 7 \rangle$ REMOVE AND REATTACH EXISTING GUARD RAIL PER DETAIL _____. 8 SHORE EXISTING FLOOR DECKING AND REMOVE EXISTING WOOD "H" FRAMING. REPLACE WITH 2x12 JOISTS AT 12" O.C. W/ 2x12 BLOCKING AT 1/3 POINTS OF JOIST SPAN. 9 INSTALL L4x4x5/16 x 2'-6" LEDGER ANGLE BELOW
- EXISTING WOOD JOIST BEARING. SECURE TO EXISTING MASONRY WALL WITH (4) 1/2" DIA ADHESIVE ANCHORS (MIN 4 1/2" EMBEDMENT). $\langle 10 \rangle$ INFILL VOID IN EXISTING MASONRY WALL WITH NON-SHRINK GROUT. PLACE SLEEVE AROUND PENETRATING
- CONDUIT. 11 NEW STRUCTURAL STEEL FRAMING FOR SUPPORT OF FAN BELOW. 12 EXISTING B2 WOOD BEAM TO BE REINFORCED PER DETAIL _____.

FRAMING LEGEND

S-3.0	= 2 7/16" NORMAL WEIGHT CONCRETE REINFORCED W/ 6x6-W2.1xW2.1 W.W.F. ON 9/16" 24 GA GALVANIZED AND THEN PRIME PAINTED NON-COMPOSITE STEEL FORM DECK (3" TOTAL THICKNESS) (+15'-8 1/2" TO MATCH EXISTING F.F.E.).
1.5WR20	= 1 1/2" 20 GA GALV WIDE RIB STEEL ROOF DECK.
TYPE 2	 = 3" LIGHTWEIGHT CONCRETE TOPPING OVER (3) 1x DECKING (6" TOTAL THICKNESS).
EX DECK TYPE 2	 = EXISTING FLOOR CONSTRUCTION CONSISTS OF 3" NORMALWEIGHT CONCRETE TOPPING OVER (3) 1x DECKING (6" TOTAL THICKNESS).
EX DECK TYPE 3	 EXISTING FLOOR CONSTRUCTION CONSISTS OF 3" NORMALWEIGHT CONCRETE TOPPING ON 1x DECKING OVER 4x DECKING (7 1/4" TOTAL THICKNESS
TYPE 4B	= 4x T&G WOOD DECKING IN TWO-SPAN CONTINUOUS LAYUP. (SOUTHERN PINE DENSE STANDARD GRADE)
(+10' - 0")	= TOP OF BEAM ELEVATION REFERENCED FROM FINISHED FIRST FLOOR ELEVATION 0'-0".
EX B1	= EXISTING WOOD BEAM. SEE WOOD BEAM SCHEDULE THIS SHEET FOR SIZE.
ML-8	= MASONRY LINTEL (BELOW). SEE DETAIL/S4.1 FOR S
L-1	= STEEL LINTEL (BELOW). SEE DETAIL/S4.1 FOR SCHE
BP-1	= STEEL BEAM BEARING PLATE. SEE DETAIL/S4.1.
W.C.J.	= WALL CONTROL JOINT. SEE DETAIL/S4.1.
****	= 8" C.M.U. WALL REINFORCED W/ #5@32" O.C. VERT. 8" BOND BEAM REINFORCED W/ (2) #5 SHALL BE PLACE AT FLOOR AND INTERMEDIATE LANDING LEVELS (SEE SECTIONS).
	= EXISTING MASONRY WALL.
WF-1	= WOOD FRAMING PER DETAIL AT EXISTING FLOOR OPENING, PENETRATION OR OTHERWISE COMPROMISED DECKING AND OR FRAMING. LOCATIO NOTED IS APPROXIMATE. CONTRACTOR SHALL DETERMINE ACTUAL LOCATION AND DIMENSIONS BASED ON CRITERIA GIVEN IN DETAIL
C12	= C12x20.7
C8	= C8x11.5
C5	= C5x6.7
LVL	= (2) 1 3/4x11 1/4 MICROLAM 1.9E (OR APPROVED EQUAL
EX F.F.E. =	(+14'-0") = EXISTING FINISHED FLOOR SPOT ELEVATION REFERENCE FROM EX FINISHED FIRST (LOW) FLOOR ELEVATION 10
	WOOD BEAM SCHEDULE

WOOD	WOOD BEAM SCHEDULE		
MARK	ACTUAL SIZE (WIDTH x DEPTH)		
B1	11x13		
B2	13x15		
B3	(5) 2x13 1/2		







PLAN NOTES

- 1. ELEVATIONS ARE SHOWN TO THE TOP OF EX CONC F.F.E. AND ARE REFERENCED FROM FINISHED FIRST FLOOR (LOW) ELEVATION 0'-0".
- 2. SEE DWG S1.0 FOR GENERAL NOTES.
- 3. EXISTING FOUNDATIONS, WALLS, FRAMING, ETC. ARE SHOWN FOR REFERENCE ONLY. CONTRACTOR SHALL FIELD VERIFY DIMENSIONS RELEVANT TO HIS WORK.
- 4. SEE DWG S2.0 FOR REFERENCE PLAN INDICATING INTERIOR AND EXTERIOR WALL ELEVATIONS.
- 5. ALL EXTERIOR STEEL FRAMING SHALL BE HOT DIPPED GALVANIZED. THIS INCLUDES STEEL LINTELS INSTALLED IN EXTERIOR WALLS.

TAG NOTES

- (1) EXISTING FLOOR DECK, STEEL JOISTS, & FLOOR LIGHTS TO BE REMOVED & REPLACED WITH FRAMING SHOWN.
- 2 NEW DOOR OPENING PER ARCH DWGS. SEE DWG S2.0 & WALL ELEVATIONS FOR ADDITIONAL INFORMATION.
- 3 > POCKET INTO EX MASONRY WALL FOR NEW JOIST BEARING PER DET _____

FRAMING LEGEND

EX S-3.0	= EX CONCRETE TOPPING SLAB OVER STEEL FORM DECK (3" TOTAL THICKNESS).
1.5WR20	= 1 1/2" 20 GA GALV WIDE RIB STEEL ROOF DECK.
W16x36 (20k)	STEEL BEAM SIZE (ASTM A992). SERVICE LOAD REACTION (KIPS) EACH END.
(+10' - 0")	= TOP OF BEAM ELEVATION REFERENCED FROM FINISHED FIRST FLOOR ELEVATION 0'-0".
EX W16	= EXISTING STEEL BEAM 16" DEEP.
ML-8	= MASONRY LINTEL (BELOW). SEE DETAIL/S4.1 FOR S0
L-1	= STEEL LINTEL (BELOW). SEE DETAIL/S4.1 FOR SCHE
BP-1	= STEEL BEAM BEARING PLATE. SEE DETAIL _/S4.1.
W.C.J.	= WALL CONTROL JOINT. SEE DETAIL _/S4.1.
\$ ** \$	= 8" C.M.U. WALL REINFORCED W/ #5@32" O.C. VERT. 8" BOND BEAM REINFORCED W/ (2) #5 SHALL BE PLACE AT FLOOR AND INTERMEDIATE LANDING LEVELS (SEE SECTIONS).
	= EXISTING MASONRY WALL.
C12	= C12x20.7
C8	= C8x11.5
C5	= C5x6.7
LVL	= (2) 1 3/4x11 1/4 MICROLAM 1.9E (OR APPROVED EQUAL)
_EX F.F.E. =	: (+14'-0")
\	= EXISTING FINISHED FLOOR SPOT ELEVATION REFERENT FROM EX FINISHED FIRST (LOW) FLOOR ELEVATION 100
SF-1	= STEEL FRAMING PER DETAIL AT EXISTING ROOF OPENING, PENETRATION OR OTHERWISE COMPROMISED DECKING AND OR FRAMING. LOCATION NOTED IS APPROXIMATE. CONTRACTOR SHALL DETERMINE ACTUAL LOCATION AND DIMENSIONS BASED ON CRITERIA GIVEN IN DETAIL











- 1. ELEVATIONS ARE SHOWN TO THE TOP OF STEEL BEAM AND ARE REFERENCED FROM FINISHED FIRST FLOOR (LOW) ELEVATION 0'-0".
- 2. SEE DWG S1.0 FOR GENERAL NOTES. 3. EXISTING FOUNDATIONS, WALLS, FRAMING, ETC. ARE SHOWN FOR REFERENCE ONLY. CONTRACTOR SHALL
- FIELD VERIFY DIMENSIONS RELEVANT TO HIS WORK.
- SEE DWG S2.0 FOR REFERENCE PLAN INDICATING INTERIOR AND EXTERIOR WALL ELEVATIONS.
- 5. ALL EXTERIOR STEEL FRAMING SHALL BE HOT DIPPED GALVANIZED. THIS INCLUDES STEEL LINTELS INSTALLED IN EXTERIOR WALLS.

TAG NOTES

- 1 EXISTING LIGHT MONITOR TO REMAIN. JACK EXISTING WOOD FRAMING UP TO 1/2" ABOVE LEVEL PRIOR TO INSTALLING STEEL FRAMING. INSTALL STEEL FRAMING PER DETAIL _____. 2 EXISTING EXTERIOR WALL TO BE ATTACHED TO EXISTING ROOF DIAPHRAGM PER DETAIL _____.
- 3 REMOVE & REPLACE DAMAGED PLY OF ROOF DECKING. NUMBER IN PARENTHESIS IS APPROXIMATE AREA.
- 4 TOP OF PARAPET WALL SHALL BE REMOVED SUCH THAT TOP OF WALL ELEV IN THE COMPLETED STRUCTURE = (+39'-0"±). INTENT IS FOR TOP OF PARAPET IN COMPLETED CONSTRUCTION TOP BE 1'-0" HIGHER THAN THE FINISHED ROOF ELEVATION AT THE RIDGE. FIELD VERIFY RIDGE ELEVATION AND ADJUST NOTED PARAPET ELEVATION ACCORDINGLY.
- 5 NEW STRUCTURAL STEEL FRAMING FOR SUPPORT OF FAN BELOW.
- 6 EX LIGHT MONITOR TO BE REMOVED. OPENING TO BE INFILLED AS NOTED.

FRAMING LEGEND

1.5WR22	= 1 1/2" 22 GA GALV WIDE RIB STEEL ROOF DECK.
+X'-X"	= TOP OF STEEL (BOTTOM OF DECK) SPOT ELEVATION. SLOPE STEEL EVENLY BETWEEN POINTS. DROP STEEL JC SUPPORT BEAMS / BEARING PLATES BY DEPTH OF JOIST (SIGNIFIED AS SD - SEAT DEPTH). WHERE NO SEAT DEPTH SPECIFIED, DEPTH SHALL BE SJI STANDARD FOR TYPE OF
	STEEL BEAM SIZE (ASTM A992).
W16x36 (2	⊥_ Ok)
(+0' - 0")	= TOP OF STEEL BEAM ELEVATION REFERENCED FROM FINISHED FIRST FLOOR ELEVATION 0'-0".
EX W16	= EXISTING STEEL BEAM 16" DEEP.
ML-8	= MASONRY LINTEL (BELOW). SEE DETAIL D/S.401 FOR SCH
L-1	= STEEL LINTEL (BELOW). SEE DETAIL E/S4.01 FOR SCHEDU
BP-1	= STEEL BEAM BEARING PLATE. SEE DETAIL F/S4.1.
W.C.J.	= WALL CONTROL JOINT. SEE DETAIL C/S4.1.
12 1/4 Г	= APPROXIMATE ROOF SLOPE. FIELD VERIFY AS NEEDED.
<u> </u>	= EXISTING MASONRY WALL.







2013 C:\L 3/4/

						SC	CHEDULE	OF R	EP/	AIRS - W	/ALL	S							
				ITEM #		DESCRIPTION					Ql	JANTITY A	ALLOWAN	CE			1		
									UNIT	S2.0 S2.1	S2.2A	S2.2B	S2.3	S2.4	S2.5	S2.6A	S2.6B	S2.7	TOT
					EX STEEL LINTEL IN TOOL CLEANED TO	EXTERIOR WALLS EXPOS BARE METAL AND PAINTE E DOWNSPOLIT PRIOR TO	D.) WALL REPAIRS AT THIS		SF										500
					CRACKS IN EXISTIN	DUT AFTER WORK AT THIS	S LOCATION IS COMPLETE	D. TYPE N	EA		<u>.</u>		0 17	0.07-	1		4.0-	105	1
					MORTAR. CRACK(S) IN EXISTI	NG STONE FOUNDATION V	WALL. CUT OUT CRACK AN	ID	LF	195	94	243	348	265	193	200	185	100	182
				<u> </u>	REPOINT WITH TYPE CRACK(S) IN EXISTI	M MORTAR.	ON WALL. ROUTE OUT CR	ACK TO		129	64			4	5			24	111 10
- K					TUCK POINT EXISTI	BY 1/2" DEEP & REPAIR WI	TYPE N MORTAR.	CATIONS.	LF	172	172	390	338	234	130	130	117	1300	298
					RECUT MASONRY J	DINT AT CORNER. INSTAL	L SEALANT PER SPECIFIC	ATIONS.	LF	40	40								80
					NEW WINDOW OPEN GALVANIZED STEEL	IING IN EXISTING MASONF LINTEL PER FRAMING PL/	RY WALL PER ARCH DWGS AN.	. INSTALL	(INCLU		STEEL BI	D)							
				9	NEW DOOR OPENIN GALVANIZED STEEL	G IN EXISTING MASONRY LINTEL PER FRAMING PL/	WALL PER ARCH DWGS.	NSTALL	(INCLU	JDE IN STRUCTURAI	STEEL BI	D)							
Ŕ					EXISTING STEEL EM TOOLED CLEANED 1	BEDDED IN EXISTING MAS O BARE METAL AND PAIN	SONRY WALL SHALL BE HA ITED.	ND	(INCLU	JDE ITEM 1 ALLOWA	NCE)								
					DAMAGED MASONR BRICK MASONRY US	Y TO BE REMOVED AND R SING TYPE N MORTAR. QL	ECONSTRUCTED WITH NE JANTITY IS NUMBER OF BF	W RICKS.	EA	219	20	45	39	27	120	5	14	50	539
	, e , e , e , e , e , e , e , e , e , e	7 1																	
K_X_X																			
		×		<u>NOTES</u> 1. 2.	S: CONTRACTOR SHALL CATEGORIZATIONS AN	DETERMINE ACTUAL LOCA	ATION OF REPAIRS ON SIT DABOVE ARE FOR CONTRA	E AND CONT ACTORS' COM	ACT THE	E ENGINEER FOR FIE	LD VERIFI	CATION P OVIDING	RIOR TO A COMMO	Making F Dn Quant	REPAIRS. TITY FOR	BID PREP	ARATION.		
				3.	BASIS OF PAYMENT: F UNIT COSTS SUBMITT	INAL PAYMENT SHALL BE ED WITH THE BID.	ADJUSTED UP OR DOWN I	FROM THE B	ASE BID	QUANTITIES INDICA	TED ABOVI	E UTILIZIN	IG ACTUA	L FIELD N	MEASURE	D QUANTI	TIES USIN	IG	
			S2.5																
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ELEVATION NOTES

 THIS PLAN IS FOR REFERENCE ONLY. SEE ELEVATIONS AND SCHEDULE REFERENCED ON THIS PLAN FOR DESCRIPTION AND APPROXIMATE LOCATION OF WORK, TO BE VERIFIED IN FIELD BY CONTRACTOR. 2. THESE NOTES APPLY TO ALL S2.# SHEETS.





Ö

B WALL (1) INTERIOR ELEVATION S2.1 3/16" = 1'-0"

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C:\Users\ara 3/4/2013 11:

______FIRST_FLOOR (LOW)

<u>ROOF</u> 37' - 0"

S2.3

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	\otimes	
× × × × × ×	\times	ROOF 37'-0"
× × × × × ×	\times	
	\times	T <u>HIRD FLOOR FRAMING PLAN</u> 27' - 9 1/2"
	XXXX	<u>MECH ROOM ROOF</u> 20'-0"
	\times	SECOND FLOOR FRAMING
	\times	
	\times	
		<u>FIRST FLOOR (LOW)</u>

B WALL (6) INTERIOR ELEVATION - PART B 3/16" = 1'-0"

	6 5.6	5	4.3 4	3	9 2	1.2
=						
A WALL (6) INTE	RIOR ELEVATION -	- PART A				

14	13	12	10 9.7	9 8.5	8	7.1 7	6 5.6	
								T <u>HIRD FLOOR FRAMING PLAN</u> 27' - 9 1/2"
								<u>MECH ROOM ROOF</u> 20' - 0" SECOND FLOOR FRAMING PLAN
								<u>PLAN</u> 15' - 8 1/2"
								<u>FIRST FLOOR (HIGH)</u> 3'- 6"
								<u>FIRST FLOOR (LOW)</u> 0"
								- <i>1</i> - 3

B WALL (7) INTERIOR ELEVATION S2.7 3/16" = 1'-0"

SECOND FLOOR FRAMING

MECH ROOM ROOF 20' - 0" - L • _____FI<u>RST FLOOR (HIGH)</u>_____3' - 6" FIRST FLOOR (LOW)

S2.7

2013 11

	LINTEL SCHEDULE										
MARK	TYPE	SIZE	JAMB REINFORCING EACH SIDE OF OPENING	REMARKS							
L-1	1	HSS16x4x5/16 + GALV PL 3/8x7	(2) #5	EXTEND JAMB REINF TO TOP OF WALL							
L-2	1	HSS16x4x3/8 + GALV PL 3/8x7	(2) #5	EXTEND JAMB REINF TO TOP OF WALL							

STEEL ROOF DECK FASTENING SCHEDULE S4.1 3/4" = 1'-0"

