GENERAL NOTES

DESIGN LOADS

WIND LOAD (PER ASCE 7-05 CHAPTER 6)

BASIC WIND SPEED	V = 90 MPH
OCCUPANCY CATEGORY	CATEGORY II
EXPOSURE CATEGORY	EXPOSURE C
IMPORTANCE FACTOR	Iw = 1.00

COMPONENTS AND CLADDING WIND DESIGN PRESSURES PER ASCE 7-05, FIGURE 6-3

ZONE 1	+10.0,	-19.53 PSF
ZONE 2	+10.0,	-32.65 PSF
ZONE 3	+10.0,	-49.24 PSF
ZONE 4	+19.53,	-21.14 PSF
ZONE 5	+19.53,	-26.09 PSF

DESIGN STRESSES

CONCRETE (STRENGTH DESIGN) MINIMUM COMPRESSIVE STRENGTH IN 28 DAYS:			
FOOTINGS	f'c =	3,000	PSI
INTERIOR SLABS ON GRADE, TUNNEL WALLS, AND CONCRETE ON METAL DECK	f'c =	4,000	PSI
CONCRETE SUBJECT TO DE-ICING SALTS	f'c =	4,500	PSI
REINFORCING BARS (ASTM A615 GRADE 60)	fy =	60,000	PSI
WELDED WIRE FABRIC (ASTM A185)	fy =	65,000	PSI
WIDE FLANGE AND TEE SHAPES DESIGNATED AS W AND WT (ASTM A992)	fy =	50,000	PSI
WIDE FLANGE AND TEE SHAPES DESIGNATED AS M, S, HP, MT AND ST (ASTM A36)	fy =	36,000	PSI
CHANNELS, ANGLES, PLATES AND BARS (ASTM A36)	fy =	36,000	PSI
STEEL FORM DECK (ASTM A653)	fy =	60,000	PSI
STEEL JOISTS (K SERIES) AS PER SJI SPECIFICATIONS	fy =	50,000	PSI MAX
	fy =	36,000	PSI MIN
SOIL BEARING PRESSURE FOR FOUNDATIONS (RESIDUAL SOIL OR COMPACTED FILL)		2,000	PSF

GENERAL

1. THE REQUIREMENTS OF THESE GENERAL NOTES APPLY UNLESS OTHERWISE NOTED ON PLANS OR IN SPECIFICATIONS.
2. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS IN THE FIELD PRIOR TO COMMENCING WORK. THE ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCIES WHICH MAY EXIST.
3. ANY DISCREPANCIES BETWEEN STRUCTURAL AND ARCHITECTURAL DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND STRUCTURAL ENGINEER.
4. DO NOT SCALE DRAWINGS.
5. THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED ON THE STRUCTURAL FRAMING. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN CAPACITY OF THE FRAMING AT THE TIME THE LOADS ARE IMPOSED.
6. SHOP DRAWINGS MUST BE CHECKED AND STAMPED BY THE CONTRACTOR PRIOR TO SUBMISSION.
7. NON-STRUCTURAL ELEMENTS OF THE BUILDING (ARCHITECTURAL FINISHES, MASONRY VENEER AND ASSOCIATED TIES, INSULATION, SHEATHING, DUCTWORK, PIPING, ETC.) ARE TYPICALLY NOT SHOWN ON THE STRUCTURAL DRAWINGS. WHERE NON-STRUCTURAL ELEMENTS ARE SHOWN ON THE STRUCTURAL DRAWINGS, THEY ARE SHOWN FOR REFERENCE AND DESIGN INTENT ONLY. NON-STRUCTURAL ELEMENTS SHALL BE CONSTRUCTED AS SHOWN ON THE ARCHITECTURAL, ELECTRICAL AND PLUMBING DRAWINGS.
8. WALL OPENINGS AND TERMINATIONS SHOWN ON THE STRUCTURAL DRAWINGS ARE DIAGRAMMATIC ONLY. WALL TERMINATIONS AND OPENING JAMBS, HEADS, AND SILLS SHALL BE CONSTRUCTED AS SHOWN ON THE ARCHITECTURAL DRAWINGS. WHERE VENEERS WRAP JAMBS, DETAIL AND FABRICATE LINTELS TO BEAR ON SOLID STRUCTURE. DO NOT BEAR LINTELS OR BEAMS ON VENEERS (BRICKS, SIDING, ETC.). IF THE ARCHITECTURAL DRAWINGS DO NOT INCLUDE DETAILS FOR ANY OF THESE CONDITIONS, CONSULT WITH ARCHITECT FOR DIRECTION.
9. EXISTING CONSTRUCTION SHOWN IS BASED ON EXISTING CONSTRUCTION DOCUMENTS AND GENERAL CONSTRUCTION PRACTICE AND IS NOT GUARANTEED TO BE TRUE OR EXACT. CONTRACTOR SHALL FIELD VERIFY ALL EXISTING CONDITIONS RELEVANT TO HIS WORK PRIOR TO CONSTRUCTION.
10. CONTRACTOR SHALL PROTECT EXISTING FACILITY AND COORDINATE HIS WORK WITH THE REMOVAL AND PROTECTION OF EXISTING STRUCTURE AND ATTACHMENTS AS REQUIRED. THIS INCLUDES BUT IS NOT LIMITED TO ELECTRICAL CONDUIT, FIXTURES, AND PIPES INCLUDING FIRE PROTECTION AND SPRINKLER HEADS. SEE MECHANICAL DRAWINGS AND SPECIFICATIONS FOR REQUIREMENTS.

CONCRETE CONSTRUCTION

1. ALL CONCRETE CONSTRUCTION TO BE IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE ACI 301-05, ACI 318-05 AND ACI DETAILING MANUAL, EXCEPT THAT CONSTRUCTION AND REMOVAL OF FORMS AND RESHORING SHALL BE INSPECTED BY THE CONTRACTOR'S ENGINEER.
2. FURNISH BAR SUPPORTS WHERE NECESSARY DURING CONSTRUCTION.
3. PROVIDE PLASTIC, PLASTIC-COATED (NOT PLASTIC-TIPPED) OR STAINLESS STEEL CHAIRS IN ALL CONCRETE EXPOSED TO VIEW IN COMPLETED STRUCTURE.
4. PROVIDE PIPE SLEEVES AND INSERTS IN CONCRETE WORK WHERE REQUIRED. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS.
5. WELDING OF REINFORCING BARS (INCLUDING TACK WELDING) IS NOT PERMITTED.
6. ALL EXPOSED CORNERS OF CONCRETE SHALL BE CHAMFERED 45 DEGREES. MINIMUM CHAMFER TO BE 1/2". CURVE THE LEADING EDGE OF STAIR TREADS TO 1/2" RADIUS.
7. REINFORCING FOR SLABS ON GROUND (IN FLAT SHEETS) SHALL BE IN THE MIDDLE OF THE SLAB EXCEPT AS OTHERWISE NOTED AND SHALL BE POSITIVELY SUPPORTED AND MAINTAINED IN THIS POSITION DURING PLACEMENT OF CONCRETE.
8. SLAB THICKNESS INDICATED OVER STEEL FORM DECK INCLUDES FORM DECK DEPTH. PROVIDE FORM DECK CONTINUOUS OVER AT LEAST THREE SUPPORTS AND WELD TO EACH SUPPORT AT MINIMUM 12" INTERVALS.
9. OBTAIN APPROVAL OF STRUCTURAL ENGINEER BEFORE LOCATING SLEEVES, HOLES, OR INSERTS IN SLABS, AND BEAMS.
10. SPLICES:
    A. LAP ALL COMPRESSION SPLICES 30 BAR DIAMETERS OF THE LARGER BAR.
    B. LAP ALL TENSION SPLICES IN ACCORDANCE WITH THE FOLLOWING TABLE. MODIFY LENGTHS AS NOTED:

BAR SIZE	CONCRETE COMPRESSIVE STRENGTH		
	3,000 PSI	4,000 PSI	5,000 PSI
#3	21"	19"	17"
#4	29"	25"	22"
#5	36"	31"	28"

1. INCREASE SPLICE LENGTH BY THE FOLLOWING:
2. NOTE: INCREASED LENGTHS ARE ACCUMULATIVE

1. HORIZONTAL TOP BARS WITH GREATER THAN 12" OF CONCRETE BELOW +30%
2. BAR SPACING LESS THAN 2 BAR DIAMETERS +50%

11. CONCRETE PROTECTION FOR REINFORCEMENT: COVER
    A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH . . . . . 3"
    B. CONCRETE EXPOSED TO EARTH OR WEATHER
       NO. 5 BAR, W31 OR D31 WIRE AND SMALLER . . . . . 1 1/2"
    C. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND
       SLABS, WALLS, AND JOISTS
       NO. 11 BAR AND SMALLER . . . . . 3/4"
       BEAMS AND COLUMNS
       PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS . . . . . 1 1/2"

ROOF, FLOOR, OR WALL OPENINGS

1. THE CONTRACTOR SHALL VERIFY AND COORDINATE THE NUMBER, SIZE, AND LOCATION OF ALL SLEEVES AND OPENINGS REQUIRED FOR MECHANICAL OR ELECTRICAL ITEMS.
2. SLEEVES AND OPENINGS SHALL BE LOCATED IN A MANNER THAT WILL MAINTAIN THE STRUCTURAL INTEGRITY OF THE ROOF, FLOOR, OR WALL SYSTEM.
3. NO STRUCTURAL ELEMENTS ARE TO BE CUT UNLESS SPECIFICALLY APPROVED BY THE ENGINEER.

LOOSE LINTEL SCHEDULE

1. THIS SCHEDULE IS FOR LINTELS OVER MASONRY OPENINGS NOT OTHERWISE SHOWN OR NOTED ON DRAWINGS, INCLUDING NON-BEARING PARTITION WALLS.
   A. ANGLES AND SUPPORT PLATES EXPOSED TO WEATHER SHALL BE HOT-DIP GALVANIZED.
   B. MINIMUM BEARING LENGTH FOR ANGLES SHALL BE 6" EACH END.
   C. PROVIDE STEEL ANGLE LINTELS ABOVE ALL DUCT PENETRATIONS 16" AND WIDER THROUGH MASONRY WALLS.
   D. FOR MULTI-WYTHE WALL CONSTRUCTION, PROVIDE LINTEL FOR EACH WYTHE PER SCHEDULE FOR GIVEN WYTHE THICKNESS.
2. FOR 8" MASONRY WALLS PROVIDE:

SPAN LIMITS	LINTEL SIZE
0" TO 6'-0"	(2) L4x3 1/2x5/16 (LLV)

STEEL CONSTRUCTION

1. STEEL DETAILING, FABRICATION, AND ERECTION SHALL CONFORM TO THE AISC SPECIFICATIONS AND CODE OF STANDARD PRACTICE, AND THE AWS STRUCTURAL WELDING CODE.
2. CONNECTIONS – WELDED OR HIGH STRENGTH BOLTED:
   A. A325-N WITH HARDENED WASHERS – USE FOR ALL CONNECTIONS.
   B. UNLESS SNUG TIGHT CONNECTIONS ARE NOTED ON THE DRAWINGS AS BEING PERMITTED, ALL BOLTS SHALL BE TIGHTENED TO FULL PRETENSIONING LOAD.
   C. UNLESS SPECIFICALLY NOTED ON THE DRAWINGS OR WITHOUT WRITTEN PERMISSION FROM THE ENGINEER, ALL BOLTS FOR THE PROJECT SHALL BE OF ONE ASTM TYPE AND ONE DIAMETER.
   D. USE STANDARD HOLES WITH THE FOLLOWING EXCEPTIONS: OVERSIZE HOLES ARE PERMITTED WHEN BOLTS ARE LOADED IN TENSION; SHORT SLOTTED HOLES ARE PERMITTED FOR SHEAR LOADING PERPENDICULAR TO THE SLOT.
   E. HARDENED WASHERS SHALL BE USED OVER ALL OVERSIZED OR SHORT-SLOTTED HOLES IN AN OUTER PLY. WHERE LONG-SLOTTED HOLES ARE USED IN AN OUTER PLY, 5/16" THICK A36 PLATE WASHERS OR CONTINUOUS BAR WITH STANDARD HOLES SHALL BE PROVIDED.
   F. WHERE REACTION IS NOTED, DEVELOP SAME. WHERE NOT NOTED, FOR NON-COMPOSITE BEAMS, CONNECTIONS SHALL DEVELOP ONE-HALF OF THE TOTAL UNIFORM LOAD CAPACITY OF THE BEAM.
   G. PREAPPROVED CONNECTION DETAILS ARE PROVIDED ON DRAWING S3.1.
   H. SINGLE PLATE SHEAR CONNECTIONS ARE NOT PERMITTED WHERE THE REACTION EXCEEDS 50 KIPS, AT FIELD APPLIED CONNECTIONS, OR CONNECTIONS TO COLUMNS (OTHER THAN AT SKEWED CONNECTIONS, MOMENT CONNECTIONS, PIPE COLUMNS, TUBE COLUMNS WITH FACE DIMENSION 4" OR LESS, OR CONNECTIONS WITH REACTIONS LESS THAN 15 KIPS).
   J. SHEAR CONNECTIONS TO VERTICAL EMBED PLATES IN CONCRETE WALLS SHALL BE DOUBLE ANGLE TYPE.
3. WELDING ELECTRODES SHALL BE E70XX EXCEPT WHERE OTHER ELECTRODES ARE REQUIRED FOR COMPATIBILITY WITH MATERIAL BEING WELDED.
4. ALL SLIP CONNECTIONS SHALL BE PROVIDED WITH A MEANS OF PREVENTING THE NUTS FROM UNTHREADING.
5. SHOP DRAWINGS ARE REQUIRED AND SHALL NOTE TYPE OF ELECTRODES, SIZE OF ALL WELDS, AND TYPE AND SIZE OF ALL BOLTS.
6. SEE SPECIFICATIONS FOR ALL PAINTING REQUIREMENTS.
7. ALL SHOP AND FIELD WELDING SHALL BE DONE BY A CERTIFIED WELDER.
8. FOR CONNECTIONS TO EXISTING CONCRETE, LOCATE THE REINFORCING BY MEANS OF A REBAR DETECTOR PRIOR TO DRILLING. ADJUST THE CONNECTION AS REQUIRED TO AVOID CUTTING ANY REINFORCING.
9. DO NOT WELD TO EXISTING STEEL WITHOUT WRITTEN APPROVAL FROM THE ENGINEER.
10. MISCELLANEOUS HANGING LOADS SUCH AS STAIR STRINGERS, PIPES, MECHANICAL UNITS, ETC., SUPPORTED BY STEEL MEMBERS SHALL HAVE THESE LOADS APPLIED IN SUCH A MANNER THAT NO TORSIONAL FORCES ARE INDUCED IN THESE MEMBERS, I.E., LOADS SHALL PASS THROUGH THE CENTERLINE OF WIDE FLANGE SECTIONS AND THROUGH THE SHEAR CENTER OF CHANNELS.

STEEL JOIST CONSTRUCTION

1. STEEL JOIST CONSTRUCTION SHALL CONFORM TO THE STANDARD SPECIFICATIONS AND CODE OF STANDARD PRACTICE FOR STEEL JOISTS AND JOIST GIRDERS OF THE STEEL JOIST INSTITUTE, LATEST EDITION.
2. JOIST BRIDGING SHOWN ON STRUCTURAL PLANS IS DIAGRAMMATIC ONLY. JOIST BRIDGING SHALL BE DESIGNED AND FURNISHED TO COMPLY WITH THE REQUIREMENTS OF THE STEEL JOIST INSTITUTE SPECIFICATION AND ANY REQUIREMENTS SHOWN IN THE CONSTRUCTION DOCUMENTS.
3. MINIMUM BEARING REQUIREMENTS ONTO STRUCTURAL STEEL UNLESS NOTED OTHERWISE SHALL BE:
   K-SERIES 2 1/2"
4. MINIMUM CONNECTION TO STRUCTURE UNLESS NOTED OTHERWISE SHALL BE:
   K-SERIES (2) 1/8"x1" FILLET WELD OR (2) 1/2"ø BOLTS

STEEL DECK CONSTRUCTION

1. STEEL DECK DETAILING, FABRICATION, AND ERECTION SHALL CONFORM TO THE LATEST, AWS STRUCTURAL WELDING CODE AND THE STEEL DECK INSTITUTE SPECIFICATIONS.
2. STEEL FLOOR DECK SHALL BE CONTINUOUS OVER A MINIMUM OF 2 SPANS.
3. DO NOT HANG OR SUPPORT ANY LOADS SUCH AS STUD WALLS, BULKHEADS, PIPES, ETC FROM STEEL ROOF DECK.

SPECIAL INSPECTION

1. SPECIAL INSPECTIONS AS DEFINED IN SECTION 1704.0 OF THE KENTUCKY BUILDING CODE ARE REQUIRED.
2. SPECIAL INSPECTIONS SHALL BE PERFORMED BY A QUALIFIED TESTING AGENCY APPROVED BY THE ARCHITECT AND THE STRUCTURAL ENGINEER AND PAID FOR BY THE OWNER.
3. THE INSPECTOR SHALL OBSERVE WORK FOR CONFORMANCE WITH THE APPROVED STRUCTURAL DRAWINGS AND SPECIFICATIONS AND PREPARE INSPECTION REPORTS STATING HIS/HER OBSERVATIONS. COPIES OF THE INSPECTION REPORTS SHALL BE SUBMITTED TO THE CONTRACTOR, THE ARCHITECT AND THE STRUCTURAL ENGINEER.
4. ALL DISCREPANCIES BETWEEN THE CONSTRUCTION DOCUMENTS AND THE WORK BEING PERFORMED SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THE DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF ARCHITECT AND THE STRUCTURAL ENGINEER PRIOR TO THE COMPLETION OF THAT PHASE OF THE WORK.
5. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL REPORT OF INSPECTIONS DOCUMENTING COMPLETION OF ALL REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS.
6. SPECIAL INSPECTIONS ARE REQUIRED FOR THE FOLLOWING WORK:

   INSPECTION OF FABRICATORS
   PERFORM SPECIAL INSPECTIONS PER SECTION 1704.2 OF THE KENTUCKY BUILDING CODE.

   STEEL CONSTRUCTION
   PERFORM SPECIAL INSPECTIONS PER SECTION 1704.3 OF THE KENTUCKY BUILDING CODE.

   CONCRETE CONSTRUCTION (INCLUDING CONCRETE REPAIRS)
   PERFORM SPECIAL INSPECTIONS PER SECTION 1704.4 OF THE KENTUCKY BUILDING CODE.

EXISTING CONDITIONS NOTE:
UNLESS OTHER ARRANGEMENTS ARE MADE PRIOR TO BID, GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING EXISTING CONDITIONS AND DIMENSIONS, INCLUDING EXISTING STRUCTURAL LAYOUTS AND ELEVATIONS, PRIOR TO SUBMITTING SHOP DRAWINGS. ACTUAL DIMENSIONS/ELEVATIONS SHALL BE MARKED ON SHOP DRAWINGS PRIOR TO SUBMITTAL TO ARCHITECT/ENGINEER. SUBCONTRACTOR'S SHALL COORDINATE WITH FIELD VERIFIED CONDITIONS RELATIVE TO THEIR WORK.

STRUCTURAL ABBREVIATIONS

ARCH	ARCHITECTURAL	LLH	LONG LEG HORIZONTAL
BOT	BOTTOM	LLV	LONG LEG VERTICAL
CLR	CLEAR	LFH	LONG FACE HORIZONTAL
CANT	CANTILEVER BEAM	LFV	LONG FACE VERTICAL
CONT	CONTINUOUS	MAX	MAXIMUM
D	DEEP	MIN	MINIMUM
DET	DETAIL	N.D.	NOMINAL DIAMETER
DWGS	DRAWINGS	N.S.	NEAR SIDE
E.F.	EACH FACE	N.T.S.	NOT TO SCALE
ELEV	ELEVATION	O.C.	ON CENTER
EMBED	MINIMUM EMBEDMENT DEPTH INTO SUBSTRATE	OPH	OPPOSITE HAND
E.W.	EACH WAY	P.A.F.	POWDER ACTUATED FASTENER
E.O.S.	EDGE OF SLAB	PL	PLATE
EXP	EXPANSION	R	RADIUS
F.S.	FAR SIDE	SIM	SIMILAR
F.F.E.	FINISHED FLOOR ELEVATION	S.O.G.	SLAB ON GRADE
GA	GAUGE	TYP	TYPICAL
GALV	GALVANIZED	VERT	VERTICAL
HORIZ	HORIZONTAL	W	WIDE
HSS	HOLLOW STRUCTURAL SECTION	W.W.F.	WELDED WIRE FABRIC

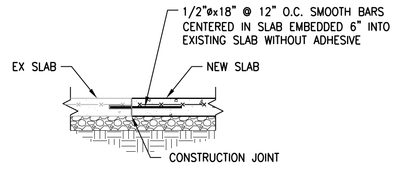
MATERIAL LEGEND

BEDROCK

NATIVE EARTH / ENGINEERED FILL

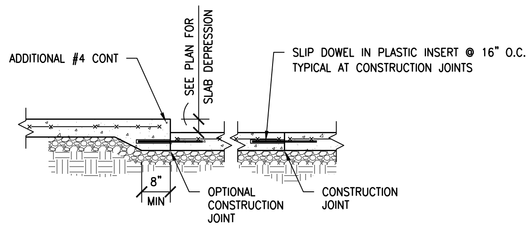
CRUSHED STONE

CONCRETE



JOINT BETWEEN EXISTING AND NEW SLAB

NOTES:
1. WELDED WIRE FABRIC SHALL BE DISCONTINUED AT CONSTRUCTION JOINTS.

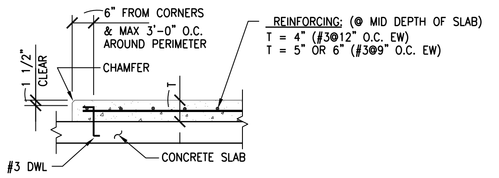


CONSTRUCTION JOINT

SLAB ON GROUND JOINT DETAIL

NOT TO SCALE

A S1.1



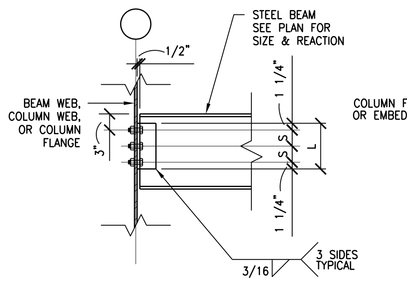
FOR NEW CONSTRUCTION

NOTES:
1. BASE SLAB PREPARATION:
   NEW CONSTRUCTION: CLEAN SLAB & WET DOWN PRIOR TO POURING PAD.
   EXIST CONSTRUCTION: SWEEP CONCRETE CLEAN, ROUGHEN SLAB & APPLY BONDING AGENT ACCORDING TO SUPPLIER'S DIRECTIONS.
2. CONCRETE MIX:– USE SAME CONCRETE AS USED ELSE WHERE ON JOB BUT NOT LESS THAN 3000 PSI (MAX 4" SLUMP)
3. CURING:– CURE FOR 7 DAYS BY EITHER OF THE FOLLOWING METHODS:
   A. COVER W/ BURLAP & KEEP CONTINUOUSLY WET.
   B. COVER W/ 6 MIL VISQUEEN. TAPE ALL JOINTS & TAPE VISQUEEN TO BASE SLAB.

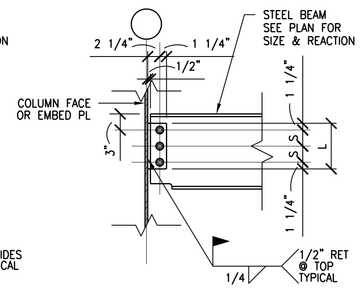
MECHANICAL HOUSEKEEPING PAD DETAIL

NOT TO SCALE

B S1.1



TYPE 1



TYPE 2

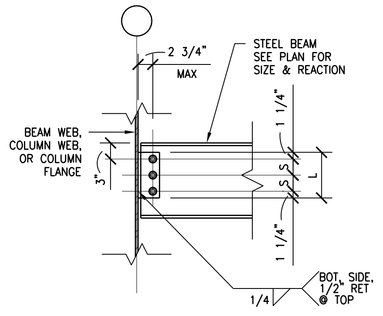
CONNECTION CAPACITY TABLE			
NO. OF BOLTS ASTM A325-N	LENGTH OF ANGLE L (INCHES)	CONNECTION CAPACITY (KIPS)	
		TYPE 1	TYPE 2
2	5 1/2	22	14
3	8 1/2	39	32
4	11 1/2	60	53

NOTES:
1. ALL CONNECTION ANGLES ASTM A36, L4x3 1/2x5/16 LLO
2. ALL BOLTS ASTM A325-N, 3/4"ø.
3. ALL WELDS E70XXX.
4. BOLT SPACING (S) = 3".
5. SHORT HORIZONTAL SLOTTED HOLES MAY BE USED.

TYPICAL DOUBLE ANGLE SHEAR CONNECTION DETAIL

NOT TO SCALE

C S1.1



CONNECTION CAPACITY TABLE		
NO. OF BOLTS ASTM A325-N	LENGTH OF ANGLE L (INCHES)	CONNECTION CAPACITY (KIPS)
2	5 1/2	9
3	8 1/2	18
4	11 1/2	28

NOTES:
1. ALL CONNECTION ANGLES ASTM A36, L4x3x3/8 SLO
2. ALL BOLTS ASTM A325-N, 3/4"ø.
3. ALL WELDS E70XX, 1/4" FILLET.
4. BOLT SPACING (S) = 3".
5. STANDARD ROUND HOLES SHALL BE USED.
6. BOTH LEGS MAY BE BOLTED WITH NO REDUCTION IN CONNECTION CAPACITY.

TYPICAL SINGLE ANGLE SHEAR CONNECTION DETAIL

NOT TO SCALE

D S1.1

11/8/2013 4:27:36 PM, mredmond

STATE OF KENTUCKY — NICHOLAS A. REDMOND — 26377 — LICENSED PROFESSIONAL ENGINEER

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REVISIONS	DATE
1	
2	
3	
4	
5	
6	
7	
8	
9	

COUNCIL OF STATE GOVERNMENTS - HEADQUARTERS BUILDING
2760 RESEARCH PARK DRIVE LEXINGTON, KY 40511

RECORD DATE: **NOV 2013**
GENERAL NOTES
DRAWING NO: **S-1.0**
DRAWN BY: **NAR**
CHECKED BY: **SCB**
COMMONWEALTH OF KENTUCKY
FINANCE AND ADMINISTRATION CABINET
DIVISION OF ENGINEERING
DEPARTMENT FOR FACILITIES MANAGEMENT
FRANKFORT, KENTUCKY
A & E FILE NO: **13013.01**
REVIEWED DIV. OF ENGR.
ENGR. FILE NO: 765-OBRA-BO01-00

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APPROVED FOR PROGRAM CONCEPT ONLY
DIVISION OF ENGINEERING _____ DATE _____
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