A. NOTES TO THE CONTRACTOR:

1. DRAWINGS REPRESENT THE DESIRED RESULT OF CONSTRUCTION. BIDDERS SHALL USE COMPLETE SETS OF BIDDING DOCUMENTS IN PREPARING BIDS; NEITHER THE, ENGINEER, OWNER NOR ARCHITECT ASSUMES RESPONSIBILITY FOR ERRORS OR MISINTERPRETATIONS RESULTING FROM THE USE OF INCOMPLETE SETS OF BIDDING DOCUMENTS.

2. THE METHODS OF CONSTRUCTION AND THE RISKS INVOLVED DURING CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL MAINTAIN THE BUILDING'S STRUCTURAL INTEGRITY AT ALL STAGES OF CONSTRUCTION.

3. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND ELEVATIONS DURING CONSTRUCTION AND REPORT TO THE ARCHITECT/ENGINEER DURING CONSTRUCTION ANY DISCREPANCIES.

4. CONTRACTOR'S PROPOSED SUBSTITUTIONS SHALL BE APPROVED BY THE ARCHITECT/ENGINEER PRIOR TO COMMENCING ANY PERTINENT WORK.

5. ALL MATERIALS AND INSTALLATIONS MUST BE TESTED/INSPECTED BY THIRD PARTY AGENCIES APPROVED BY THE STRUCTURAL ENGINEER, BUT EMPLOYED BY THE OWNER. SEE INDIVIDUAL SPECIFICATION SECTIONS FOR SPECIFIC REQUIREMENTS. OWNER SHALL SUBMIT A LIST OF PROPOSED INSPECTORS AT THE PRE-CONSTRUCTION MEETING; LIST SHALL INCLUDE CREDENTIALS OF INSPECTOR(S) WHO WILL ACTUALLY PERFORM THE WORK.

6. SPECIAL INSPECTIONS:

   a. A SPECIAL INSPECTIONS PROGRAM IS REQUIRED FOR THIS PROJECT. THE STRUCTURAL ENGINEER OF RECORD, BFMJ, MUST APPROVE ALL SPECIAL INSPECTORS FOR STRUCTURAL WORK IN WRITING PRIOR TO BEGINNING ANY FIELD CONSTRUCTION WORK.

   b. THE SPECIAL INSPECTOR (SI) THAT OVERSEES EACH SPECIAL INSPECTION PROGRAM AREA MUST BE A REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF KENTUCKY. HE AND HIS INSPECTION AGENTS SHALL HAVE APPROPRIATE EXPERIENCE AND TRAINING IN THE AREA OF WORK THEY WILL BE PROVIDING SERVICES FOR, INCLUDING THE ABILITY TO READ AND INTERPRET THE CONTRACT DOCUMENTS, INCLUDING CONTRACT DRAWINGS, SPECIFICATIONS AND FIELD ADDENDUMS.

   c. SPECIAL INSPECTIONS SHALL BE IN ACCORDANCE WITH CHAPTER 17 OF THE 2007 KENTUCKY BUILDING CODE (KBC)

   d. ALL MATERIALS SUPPLIERS ARE TO SUBMIT ALL NECESSARY PAPERWORK OR HIRE A SPECIAL INSPECTOR TO MEET THE REQUIREMENTS FOR INSPECTION OF FABRICATORS LISTED IN CHAPTER 17 OF THE 2007 KBC, SECTION 1704.2.

   e. THE OWNER IS TO ENGAGE A SPECIAL INSPECTOR OR INSPECTORS THAT WILL OBSERVE AND REPORT ON THE CONSTRUCTION PRACTICES.

   f. THE CONTRACTOR OR HIS SUBCONTRACTORS ARE TO ENGAGE THE TESTING AGENCIES FOR ALL MATERIAL TESTING REQUIRED IN THEIR SPECIFICATIONS OF WORK.

   g. DUTIES OF THE SPECIAL INSPECTOR:

      1) SPECIAL INSPECTORS AND THE AUTHORIZED AGENT(S) SHALL OBSERVE DESIGNATED WORK TO VERIFY CONFORMANCE WITH THE APPROVED DRAWINGS AND SPECIFICATIONS.

      2) THE **SI** SHALL INFORM ALL PERTINENT PARTIES, INCLUDING THE SER, OF ANY DEVIATIONS FROM COMPLIANCE WITH THE CONTRACT DOCUMENTS IMMEDIATELY.

      3) THE **SI** IS TO PROVIDE PERIODIC INTERIM REPORTS TO THE SER, BFMJ, AND FILE A FINAL REPORT OF SPECIAL INSPECTIONS AT THE COMPLETION OF THE PROJECT TO THE SER AND BUILDING OFFICIAL.

   h. THE FOLLOWING TYPES OF STRUCTURAL WORK REQUIRE SPECIAL INSPECTION: (REFER TO BUILDING CODE, SPECIFICATIONS, AND SPECIAL INSPECTION LETTER FOR DETAILED INSPECTION REQUIREMENTS):

      FABRICATORS
      STRUCTURAL STEEL WORK
      CONCRETE WORK
      MASONRY WORK

B. DESIGN CRITERIA

THIS STRUCTURE HAS BEEN DESIGNED ACCORDING TO THE 2007 KENTUCKY BUILDING CODE AND FOR THE SPECIFIC LOADS THAT ARE LISTED BELOW.

1. ROOF LOADS

   a. DEAD LOAD = 15 PSF + STRUCTURAL FRAMING

   b. MECHANICAL, CEILING, LIGHTING = 10 PSF

   c. LIVE LOAD = 20 PSF

   d. SNOW LOADS:

      GROUND SNOW LOAD (Pg) = 20 PSF

      FLAT-ROOF SNOW LOAD (P_f) = 20 PSF

      EXPOSURE FACTOR (Ce) = .9

      IMPORTANCE FACTOR (I_S) = 1.0

      THERMAL FACTOR (C_t) = 1.0

2. FLOOR LOADS

   a. SLAB ON GRAD = 150 PSF

   b. LATERAL LOADS:

      1) WIND LOADS:

         BASIC WIND SPEED = 90 MPH

         IMPORTANCE FACTOR (I_w) = 1.0

         BUILDING CATEGORY = II

         EXPOSURE = C

         INTERNAL PRESSURE COEFFICIENT (GC_{pi}) = +.18 and -.18

         MAIN WINDFORCE DESIGN PRESSURE (P) = 17 PSF

         COMPONENTS AND CLADDING DESIGN PRESSURE PER 2007 KBC FIGURE 1609.6(2):

         ZONE 1 = +6.7 PSF or -12.5 PSF
         ZONE 2 = +7.7 PSF or -21.4 PSF
         ZONE 3 = +8.4 PSF or -34.3 PSF
         ZONE 4 = +12.4 PSF or -13.6 PSF
         ZONE 5 = +13 PSF or -16.5 PSF

      2) SEISMIC LOADS:

AN EARTHQUAKE DESIGN ANALYSIS WAS PERFORMED ON THE BUILDING STRUCTURE. THESE DESIGN FORCES DID NOT GOVERN THE LATERAL DESIGN OF THE BUILDING STRUCTURE.

THE FOLLOWING EARTHQUAKE DESIGN DATA WAS USED FOR THIS DESIGN ANALYSIS:

MAPPED SPECTRAL RESPONSE ACCELERATION S_S = 0.194

MAPPED SPECTRAL RESPONSE ACCELERATION S_{1} = = 0.078

DESIGN SPECTRAL ACCELERATION FOR SHORT PERIOD (S_{DS}) = 0.155

DESIGN SPECTRAL ACCELERATION FOR 1-SECOND PERIOD (S_{D1}) = 0.088

SEISMIC USE GROUP = II

SEISMIC DESIGN CATEGORY = B

SITE CLASS TYPE = C

BASIC STRUCTURAL SYSTEM AND SEISMIC-RESISTING SYSTEM:

RESPONSE MODIFICATION FACTOR (R) = 2

SEISMIC RESPONSE COEFFICIENT (Cs) = 0.0776

DESIGN BASE SHEAR = 3.88 KIPS

ANALYSIS PROCEDURE: = EQUIVALENT LATERAL FORCE PROCEDURE (OR MODAL ANALYSIS PROCEDURE OR SIMPLIFIED PROCEDURE).

C. FOUNDATION, FILLING, AND EXCAVATION (SOIL REPORT)

THE FOLLOWING DESIGN INFORMATION HAS BEEN OBTAINED FROM A SUBSURFACE REPORT PREPARED BY ALT & WITZIG ENGINEERING, INC. DATED APRIL 9, 2012.

1. SOIL BEARING CAPACITY:

   a. ISOLATED FOOTINGS = 1500 PSF

   b. CONTINUOUS FOOTINGS = 1500 PSF

2. FOR INFORMATION ON FILLING AND EXCAVATION AND BACKFILLING, SEE GEOTECHNICAL REPORT DATED APRIL 9, 2012 AS PREPARED BY ALT & WITZIG ENGINEERING, INC.

3. ALL FILL BELOW SLABS ON GRADE SHALL BE COMPACTED TO 98% OF STANDARD PROCTOR DENSITY, ASTM D698 98%, AT +/- 2% OF OPTIMUM MOISTURE CONTENT (O.M.C.). ALL FILL IN THE BEARING ZONE BELOW FOOTINGS SHALL BE COMPACTED TO 100% OF S.P.D. AT +/- 2% OF O.M.C.

4. A MINIMUM OF ONE FIELD DENSITY TEST SHOULD BE PERFORMED FOR THE BUILDING AREA WITH A MINIMUM OF TWO TESTS FOR EACH LAYER OF FILL. FOR BACKFILL OF DITCHES OR TRENCHES, ONE DENSITY TEST SHOULD BE PERFORMED FOR EACH 10 CUBIC YARDS (IN PLACE) OF BACKFILL MATERIAL, UNLESS OTHERWISE NOTED.

5. PROVIDE A 2'-0" THICK MINIMUM COMPACTED SOIL "CUSHION" BETWEEN BOTTOM OF FOOTING AND TOP OF ROCK AND BOTTOM OF FOOTING.

6. FOOTINGS SHALL BEAR ON UNDISTURBED (IN-SITU) SOIL [AND ENGINEERED FILL] AND SHALL BE PLACED ON A LEVEL SURFACE.

D. CAST IN PLACE CONCRETE

1. PRIOR TO FABRICATION, SUBMIT SHOP DRAWINGS FOR FABRICATION, BENDING AND PLACEMENT OF CONCRETE REINFORCEMENT. COMPLY WITH ACI 315 "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" SHOWING BAR SCHEDULES, STIRRUP SPACING, DIAGRAMS OF BENT BARS, AND ARRANGEMENT OF CONCRETE REINFORCEMENT. INCLUDE SPECIAL REINFORCEMENT REQUIRED AND OPENINGS THROUGH CONCRETE STRUCTURES.

2. SUBMIT LABORATORY TEST REPORTS FOR CONCRETE MATERIALS AND MIX DESIGN TEST AS SPECIFIED.

3. ALL CONCRETE SHALL DEVELOP 4000 PSI COMPRESSIVE STRENGTH IN 28 DAYS.

4. REINFORCING BARS SHALL BE DEFORMED AND SHALL CONFORM TO ASTM A615, Fy = 60 KSI. REINFORCING BARS INDICATED TO BE WELDED SHALL CONFORM TO ASTM A706. WELDED WIRE REINFORCEMENT SHALL CONFORM TO ASTM A185.

5. SPLICES IN CONTINUOUS VERTICAL OR HORIZONTAL REINFORCING BARS SHALL BE PER LATEST EDITION OF ACI 318 OR 50 BAR DIAMETER LAP SPLICE WHICHEVER IS GREATER UNLESS NOTED OTHERWISE AND BARS SHALL BE EITHER CONTINUOUS OR SPLICED WITH CORNER BARS AT CORNERS OR TEES (SEE STANDARD DETAILS).

6. WELDED WIRE REINFORCEMENT (WWR) SHALL CONFORM TO ASTM A185. ALL MESH/WWR MUST BE SUPPLIED IN FLAT SHEETS AND ALL WWR SHALL BE PROPERLY SUPPORTED WITH STANDARD CHAIRS FOR CONCRETE ON METAL DECK OR CONCRETE BRICK FOR SLAB-ON-GRADE. HOOKING AND LIFTING WILL NOT BE PERMITTED (FOR ELEVATED OR SLAB-ON-GRADE).

7. CLEARANCES BETWEEN REINFORCING BARS AND CONCRETE SURFACES SHALL BE AS FOLLOWS:

	MINIMUM COVER, IN
a. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3
b. CONCRETE EXPOSED TO EARTH OR WEATHER:	
#6 THROUGH #18 BARS	2
#5 BAR AND SMALLER	1-1/2
c. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:	
SLABS, WALLS: #11 AND SMALLER	1
BEAMS, COLUMNS: PRIMARY REINFORCEMENT, TIES AND STIRRUPS	1-1/2

8. PROVIDE CONTROL OR CONSTRUCTION JOINTS IN SLAB ON GRADE WHERE INDICATED ON DRAWINGS OR AT MAXIMUM SPACING OF14' EACH WAY, UNLESS NOTED OTHERWISE. COORDINATE ANY JOINT LOCATION CHANGES WITH ARCHITECT.

E. CONCRETE MASONRY UNITS

1. HORIZONTAL WIRE REINFORCING SHALL BE (9) GAUGE CONTINUOUS LADDER TYPE UNITS COMPLYING WITH ASTM A-82 AND SPACED @ (16") OC VERTICALLY, UNLESS NOTED OTHERWISE.

2. UNLESS NOTED OTHERWISE, ALL VERTICAL REINFORCING SHALL BE #5 BARS AT (32") OC PLUS ONE BAR AT EACH DOOR AND WINDOW JAMB, HEAD AND SILL. JAMB BARS SHALL RUN FULL HEIGHT WHILE HEAD AND SILL BARS SHALL EXTEND TWO FEET BEYOND EDGE OF OPENING. ALSO, PROVIDE 2 - #5'S AT EACH CORNER AND WHERE ADDITIONAL REINFORCING IS INDICATED.

3. PROVIDE DOWELS BETWEEN ALL MASONRY AND FOUNDATION WALLS, AS SHOWN.

4. SPLICES IN CONTINUOUS VERTICAL OR HORIZONTAL REINFORCING BARS SHALL BE 48 BAR DIAMETER LAP SPLICES. ALL SPLICES MUST BE SECURELY TIED TO DOWELS, CORNER BARS OR SPLICE BARS UNLESS OTHERWISE NOTED.

5. FILL CELLS OF BLOCK AT VERTICAL REINFORCING AND BOND BEAMS AND WHERE INDICATED WITH 3/8" (MAX.) LARGE AGGREGATE CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI AT 28 DAYS AND SHALL COMPLY WITH ASTM C476.

6. ALL CELLS OF BLOCK BELOW GRADE SHALL BE GROUTED SOLID.

7. CONTRACTOR SHALL REVIEW ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR OTHER MISC. STEEL LINTEL(S) NOT SHOWN AND INCLUDE IN BASE BID.

F. STRUCTURAL STEEL

1. ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM A36 UNLESS NOTED OTHERWISE.

2. ANCHOR BOLTS SHALL CONFORM TO ASTM F1554 GR 36.

3. ALL HEADED STUD ANCHORS SHALL BE MANUFACTURED BY "NELSON STUD" OR APPROVED EQUAL.

4. ALL WELDS TO BE MADE WITH E70XX RODS AND WELDING IS TO CONFORM TO LATEST AWS CODE.

5. STEEL PIPE COLUMNS SHALL CONFORM TO ASTM A501, Fy=36 KSI OR ASTM A53, TYPE E OR S, GRADE B.

   a. WHERE SPECIFIED NUMBER EXCEEDS NUMBER PERMITTED BY MINIMUM SPACING IN ONE ROW, USE EXCESS STUDS TO MAKE PAIRS OF STUDS AT MINIMUM SPACING NEAR ENDS OF BEAM.

6. IF LONGITUDINAL STUD SPACING EXCEEDS 18", PLUG WELD DECK TO BEAM HALFWAY BETWEEN STUDS. NOTE PROVIDE ADDITIONAL PLUG WELDS AS REQUIRED IF RESULTING SPACING BETWEEN WELD AND STUD IS MORE THAN 18".

G. FRAMING LUMBER (AND PLYWOOD)

1. ALL LUMBER SHALL BE GRADED IN ACCORDANCE WITH NFPA STANDARDS.

   a. ROOF RAFTERS AND FLOOR JOIST SHALL BE SOUTHERN YELLOW PINE, GRADE NO. 2 AT 19% MOISTURE CONTENT WITH ALLOWABLE SINGLE MEMBER BENDING STRESS, Fb, OF 1200 PSI AND A MODULUS OF ELASTICITY, E, OF 1,600,000 PSI.

2. WOOD CONNECTORS SHOWN ON THE DRAWINGS SHALL BE SIMPSON STRONG-TIE CONNECTORS AS MANUFACTURED BY THE SIMPSON CO. OR APPROVED EQUAL.

   a. ALL TRUSS TO WALL CONNECTIONS SHALL BE MADE WITH WIND ANCHORS – TYPE H2.5 OR H3 (BY SIMPSON CO.)

3. PROVIDE NAILS AND BOLTS AS INDICATED ON THE DRAWINGS.

4. ALL PLYWOOD SHALL BE STRESS RATED AT (32/16) EXTERIOR GRADE EXPOSURE 1, (C-DX), WITH THICKNESS AS INDICATED ON THE DRAWINGS.

5. PLYWOOD ON ROOF SHALL BE PLACED WITH LONG DIMENSION ACROSS TRUSSES AND STAGGERED SO CONTINUOUS PANEL JOINTS OCCUR ONLY IN LONG DIRECTION, PERPENDICULAR TO THE SPAN OF THE TRUSSES. NAIL PLYWOOD WITH 10d COMMON OR GALVANIZED BOX NAILS. NAILS TO BE SPACED AT 6" OC AT SUPPORTED EDGES OF PANEL AND AT 12" OC AT INTERMEDIATE SUPPORT MEMBERS, UNLESS NOTED. SPACE ALL ROOF PANELS WITH 1/8" SPACE BETWEEN (ALL AROUND), USING PANEL CLIPS (H-CLIPS) AT 12'' MAXIMUM SPACING.

H. LAMINATED WOOD

1. ALL LVL'S SHALL BE LAMINATED VENEER LUMBER AND SHALL BE DETAILED AS SHOWN ON THE PLANS AND SHALL PROVIDE MINIMUM ALLOWABLE DESIGN VALUES OF 2800 PSI IN BENDING (Fb), AND 2,000,000 PSI IN MODULUS OF ELASTICITY, FOR DRY CONDITION OF SERVICE. ALL LVL MATERIAL MUST BE KEPT DRY DURING ALL OF CONSTRUCTION.

I. NOTES FOR WOOD TRUSSES

1. TRUSS MANUFACTURER SHALL DESIGN AND FURNISH ALL TRUSSES, TRUSS COMPONENTS, AND TRUSS TO TRUSS CONNECTIONS.

2. WOOD TRUSSES SHALL BE DESIGNED BY THE SUPPLIER TO SUPPORT THEIR SELF-WEIGHT, PLUS THE SUPERIMPOSED LOADS LISTED.

   a. TOP CHORD:

      1) DEAD LOAD = 15 PSF

      2) LIVE LOAD = 25 PSF+ DRIFTING

   b. BOTTOM CHORD SUPERIMPOSED LOADS

      1) DEAD LOAD = 10 PSF

3. TRUSS SUPPLIER TO SHOW ALL BRACING REQUIREMENTS FOR TRUSS DURING CONSTRUCTION AND PERMANENT PHASE. IN ADDITION ERECTOR SHALL THOROUGHLY FAMILIARIZE HIMSELF WITH BCSI 1-03 "GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING AND BRAZING OF METAL PLATE CONNECTED WOOD TRUSSES".

4. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS OF WOOD TRUSSES PRIOR TO FABRICATION. INDICATE SPECIES AND STRESS GRADE OF LUMBER TO BE USED AND DETAILS OF METAL CONNECTORS TO BE USED AT ALL JOINTS. SHOW PITCH, SPAN AND LOCATION OF TRUSSES. PROVIDE LARGE SCALE DETAILS OF TYPICAL CONNECTION AND ANCHORAGES.

5. CONTRACTOR SHALL PROVIDE TRUSS MANUFACTURER'S DESIGN AND ENGINEERING DATA FOR THE REQUIRED TRUSSES, TRUSS COMPONENTS AND TRUSS-TO-TRUSS CONNECTION. INFORMATION SHALL INCLUDE LOADING DIAGRAMS, STRESS VALUES, CONNECTOR TYPE AND SIZES. ALL INFORMATION SHALL BEAR THE SEAL OF A QUALIFIED LICENSED PROFESSIONAL ENGINEER IN THE STATE WORK IS BEING DONE.

6. THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION. THE FOLLOWING RECOMMENDATIONS ARE PRESENTED ONLY AS A GUIDE FOR THE USE OF A QUALIFIED BUILDER OR ERECTOR. BUELL FRYER MCREYNOLDS JAHED, INC. EXPRESSLY DISCLAIMS ANY RESPONSIBILITY FROM THE USE OF THIS IMPLEMENTATION. CONTRACTOR SHALL PROVIDE TEMPORARY AND PERMANENT BRACING AT 3 LOCATIONS ON THE TRUSSES AS PRESCRIBED BY MANUFACTURER AND BCSI 1-03: A) BOTTOM CHORD, B) WEB MEMBERS, AND C) TOP CHORD. TEMPORARY BRACING MAY REMAIN IN PLACE AS PERMANENT BRACING IF THESE INSTRUCTIONS ARE FOLLOWED AND IF NAILED SO THAT THE NAILS ARE LOADED LATERALLY AND NOT IN WITHDRAWAL.

   a. BOTTOM CHORD PLANE: INSTALL CONTINUOUS LATERAL BRACING ACROSS THE ENTIRE WIDTH OF THE BUILDING AND PERPENDICULAR TO THE TRUSSES. THE FIRST BRACE SHALL BE LOCATED AT AND PARALLEL TO THE END LINE OF THE TRUSS SPAN. ADDITIONAL BRACING SHALL BE LOCATED PARALLEL TO THE FIRST BRACE AND AT 8' - 0" TO 10' - 0" INTERVALS. THESE SHALL BE NAILED TO THE TOP SIDE OF THE BOTTOM CHORD AND SHALL OVERLAP AT LEAST TWO TRUSSES WHERE CONNECTED. ONE COMPLETE BAY OF FLAT DIAGONAL BRACING SHALL BE APPLIED AT THE FRONT AND THE EXTREME REAR SO AS TO FORM (TWO) FLAT OR HORIZONTAL TRUSSES. THIS BRACING SHALL BE AT APPROXIMATELY 45 DEGREES TO THE LATERAL BRACING AND RUN BETWEEN THE BEARING WALL AND NEAREST LATERAL BRACE.

   b. WEB MEMBER PLANE: INSTALL BRACING ON ALL WEB MEMBERS AS SHOWN ON TRUSS SHOP DRAWINGS. ADDITIONAL BRACES SHALL BE NAILED TO THESE WEB MEMBERS AND AT APPROXIMATELY 45 DEGREE ANGLES TO THE WEB MEMBERS. DIAGONAL BRACES SHALL BE BETWEEN 12'-0" AND 16'-0" IN LENGTH AND SHALL BE REPEATED AT APPROXIMATELY 20'-0" INTERVALS ALONG THE SPACING OF THE TRUSSES.

   c. TOP CHORD PLANE: TOP CHORDS WILL BE SUFFICIENTLY BRACED WHEN PLYWOOD ROOF SHEATING IS PROPERLY INSTALLED. TEMPORARY ERECTION BRACING SHALL BE INSTALLED AS SPECIFIED PER TRUSS DRAWINGS.

   d. IN ACCORDANCE WITH PREVIOUSLY MENTIONED BCSI 1-03, ERECTOR SHALL INSTALL GROUND BRACING SO AS TO PROPERLY STABILIZE THE FIRST TRUSS ERECTED.

   e. MINIMUM SIZE FOR DIAGONAL AND LATERAL WOOD BRACES SHALL BE 2" x 4", WITH A MINIMUM OF TWO SIXTEEN (16) PENNY NAILS AT THE JUNCTION OF EACH BRACE AND EACH TRUSS MEMBER.

LEGEND

ENGINEERED FILL OR UNDISTURBED SOIL
BEDROCK
CONCRETE
GROUT
CRUSHED STONE
STEEL
EXISTING
BLOCK
BRICK
PLYWOOD OR ENGINEERED LUMBER
PRECAST
WOOD CROSS SECTION
DENSE GRADE AGGREGRATE

RX WATER STOP
KEYWAY - 2" x 4" x CONT. (TYP)
MOMENT CONNECTION
[XX] INDICATES NUMBER OF STUD ANCHORS EQUALLY SPACED OVER ENTIRE LENGTH OF BEAMS.
DENOTES DIRECTION OF DECK SPAN
XX ELEVATION NOTE
XX PLAN NOTE
XX REVISIONS

ABBREVIATIONS

(FOR STRUCTURAL DRAWINGS ONLY)

&	AND
@	AT
AB	ANCHOR BOLT
AFF	ABOVE FINISHED FLOOR
ADH.	ADHESIVE
ALT.	ALTERNATE
ALUM.	ALUMINUM
ANCH.	ANCHOR
APPROX.	APPROXIMATE
ARCH.	ARCHITECT/ARCHITECTURAL
BB	BACK TO BACK
BBE	BOTTOM OF BEAM ELEVATION
BDSE	BOTTOM OF DRILL SHAFT ELEVATION
BFE	BOTTOM OF FOOTING ELEVATION
BKSE	BRICK/BLOCK SEAT ELEVATION
BLDG.	BUILDING
BLKG.	BLOCKING
BM.	BEAM
BOT.	BOTTOM
BPE	BOTTOM OF PILE ELEVATION
BRG.	BEARING
BSE	BOTTOM OF STEEL ELEVATION
BWE	BOTTOM OF WALL ELEVATION
CB	CONCRETE BLOCK
CJ	CONTROL JOINT
C	CENTER LINE
CL.	CLEAR
CMU	CONCRETE MASONRY UNIT
COL.	COLUMN
COMP.	COMPOSITE
CONC.	CONCRETE
CONN.	CONNECTION
CONST.	CONSTRUCTION
CONT.	CONTINUOUS
CONTR.	CONTRACTOR
COOR.	COORDINATE
CSJ	CONSTRUCTION JOINT
CS	COLUMN STRIP
DB.	BAR DIA.
DBA	DEFORMED BAR ANCHORS
DBL.	DOUBLE
DTL.	DETAIL
DIA. or Ø	DIAMETER
DIAG.	DIAGONAL
DIM.	DIMENSION
DL	DEAD LOAD
DN.	DOWN
DP	DRILLED PIER
DS	DRILLED SHAFT
DWL.	DOWELS
DWG.	DRAWING'S
EA.	EACH
EB	ERECTION BOLT
EF	EACH FACE
EJ	EXPANSION JOINT
ELEV.	ELEVATION
EMB.	EMBEDMENT
ENGR.	ENGINEER
EQ.	EQUAL
EW	EACH WAY
EXIST.	EXISTING
EXP.	EXPANSION
EXT.	EXTERIOR
FDN.	FOUNDATION
FFE	FINISHED FLOOR ELEVATION
FIN.	FINISHED
FL.	FLOOR
FLGS.	FLANGES
FRMG.	FRAMING
FT.	FOOT
FTG.	FOOTING
FV	FIELD VERIFY
GA.	GAUGE
GALV.	GALVANIZED
GB	GRADE BEAM
GC	GENERAL CONTRACTOR
GR.	GRADE
HC	HOLLOW CORE
HS	HEADED STUDS
HSB	HIGH STRENGTH BOLTS
HT.	HEIGHT
HORZ.	HORIZONTAL
HSS	HOLLOW STRUCTURAL SECTION
JBE	JOIST BEARING ELEVATION
JT.	JOINT
KLF	KIPS PER LINEAR FOOT
KSF	KIPS PER SQUARE FOOT
LB.	POUND
LL	LIVE LOAD
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LT. WT.	LIGHT WEIGHT
LVL	LAMINATED VENEER LUMBER
MAS.	MASONRY
MAX.	MAXIMUM
MECH.	MECHANICAL
MFG.	MANUFACTURER
MIN.	MINIMUM
MISC.	MISCELLANEOUS
MO	MASONRY OPENING
MOM.	MOMENT
MS	MIDDLE STRIP
MTL.	METAL
NIC	NOT IN CONTRACT
NO. or #	NUMBER
NTS	NOT TO SCALE
OC	ON CENTER
OPNG.	OPENING
OSF	OUTSIDE FACE
PCF	POUNDS PER CUBIC FOOT
PC	PILE CAP
PLF	POUNDS PER LINEAR FOOT
PLYWD.	PLYWOOD
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PSL	PARALLEL STRAND LUMBER
R	RADIUS
RAD.	RADIUS
REINF.	REINFORCEMENT
REQD.	REQUIRED
RET.	RETAINING
SCHED.	SCHEDULE
SECT.	SECTION
SER	STRUCTURAL ENGINEER OF RECORD
SHT.	SHEET
SIM.	SIMILAR
SP.	SPACE
STIFF.	STIFFENER
STD.	STANDARD
STL.	STEEL
STRUC.	STRUCTURAL
TBE	TOP OF BEAM ELEVATION
TOC	TOP OF COLUMN ELEVATION
TCE	TOP OF CONCRETE ELEVATION
TDSE	TOP OF DRILL SHAFT ELEVATION
TFE	TOP OF FOOTING ELEVATION
TJE	TOP OF JOIST ELEVATION
TPCE	TOP OF PILE CAP ELEVATION
TRE	TOP OF ROCK ELEVATION
TSE	TOP OF SLAB ELEVATION
TWE	TOP OF WALL ELEVATION
TYP.	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VERT.	VERTICAL
W/	WITH
WP	WORKING POINT
WWR	WELDED WIRE REINFORCEMENT

NOT FOR CONSTRUCTION

bfmj
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CANNONDESIGN

NKU NORTHERN KENTUCKY UNIVERSITY

Northern Kentucky University - Intramural Field and Support Building

#	Revision	Date
1		Date 1

Drawing Name:	GENERAL NOTES
NKU Project Number:	
Project Number:	11107
Date:	7-11-12
Drawn By:	Author
Checked By:	Checker

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